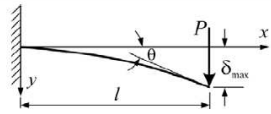
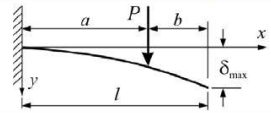
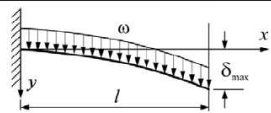
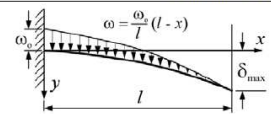
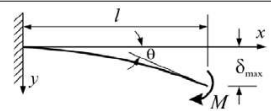
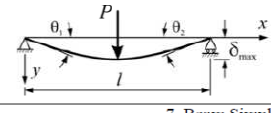
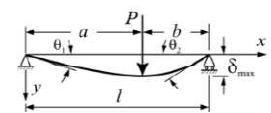
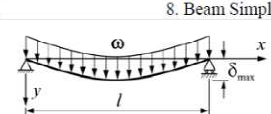
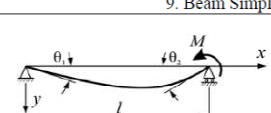
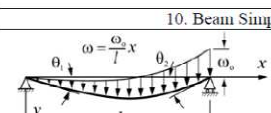
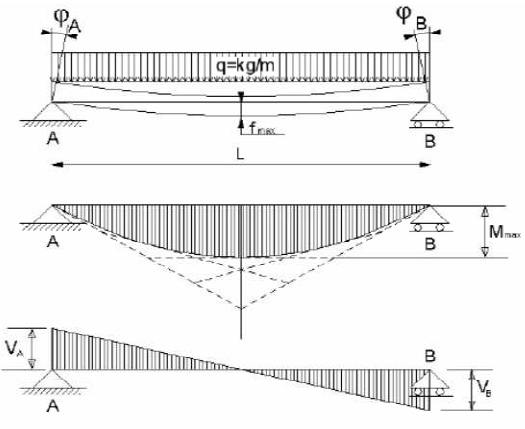
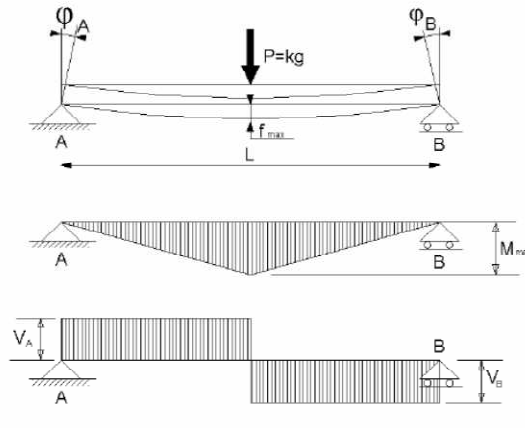


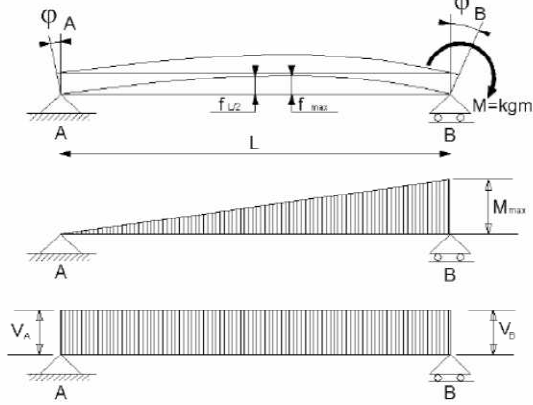
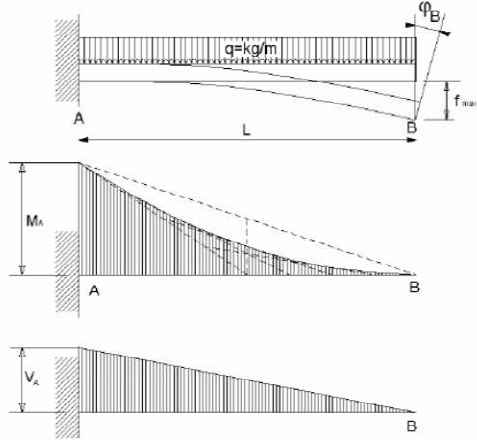
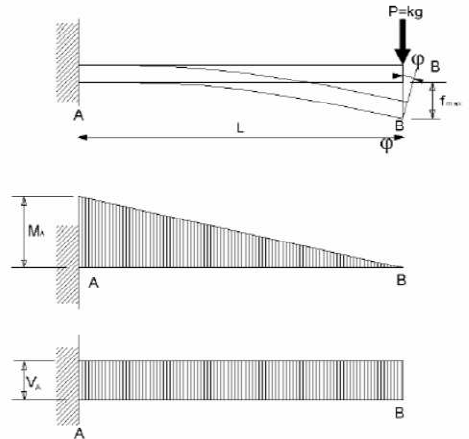
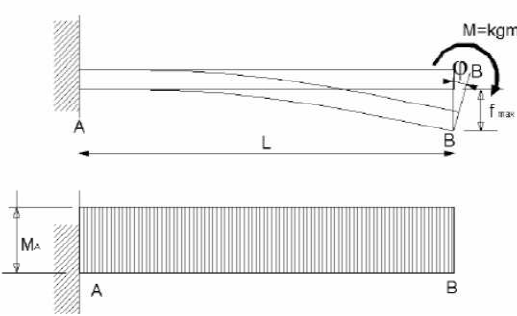
BEAM DEFLECTION FORMULAE

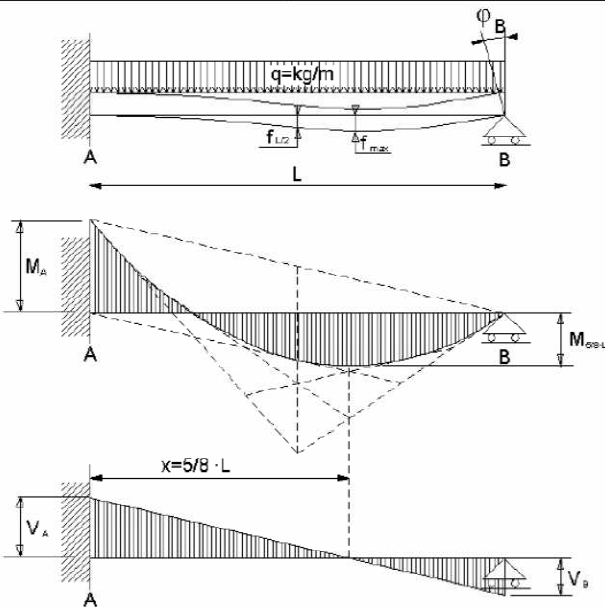
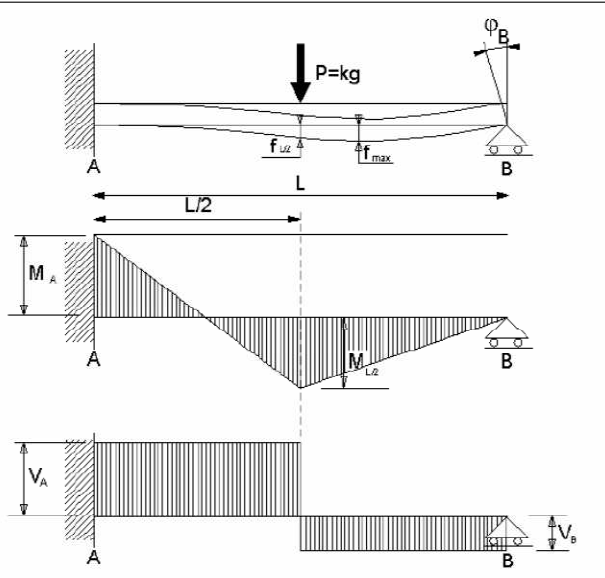
BEAM TYPE	SLOPE AT FREE END	DEFLECTION AT ANY SECTION IN TERMS OF x	MAXIMUM DEFLECTION
1. Cantilever Beam – Concentrated load P at the free end			
	$\theta = \frac{Pl^2}{2EI}$	$y = \frac{Px^2}{6EI}(3l-x)$	$\delta_{\max} = \frac{Pl^3}{3EI}$
2. Cantilever Beam – Concentrated load P at any point			
	$\theta = \frac{Pa^2}{2EI}$	$y = \frac{Px^2}{6EI}(3a-x)$ for $0 < x < a$ $y = \frac{Pa^2}{6EI}(3x-a)$ for $a < x < l$	$\delta_{\max} = \frac{Pa^2}{6EI}(3l-a)$
3. Cantilever Beam – Uniformly distributed load ω (N/m)			
	$\theta = \frac{\omega l^3}{6EI}$	$y = \frac{\omega x^2}{24EI}(x^2 + 6l^2 - 4lx)$	$\delta_{\max} = \frac{\omega l^4}{8EI}$
4. Cantilever Beam – Uniformly varying load: Maximum intensity ω_0 (N/m)			
	$\theta = \frac{\omega_0 l^3}{24EI}$	$y = \frac{\omega_0 x^2}{120EI}(10l^3 - 10l^2x + 5lx^2 - x^3)$	$\delta_{\max} = \frac{\omega_0 l^4}{30EI}$
5. Cantilever Beam – Couple moment M at the free end			
	$\theta = \frac{Ml}{EI}$	$y = \frac{Mx^2}{2EI}$	$\delta_{\max} = \frac{Ml^2}{2EI}$

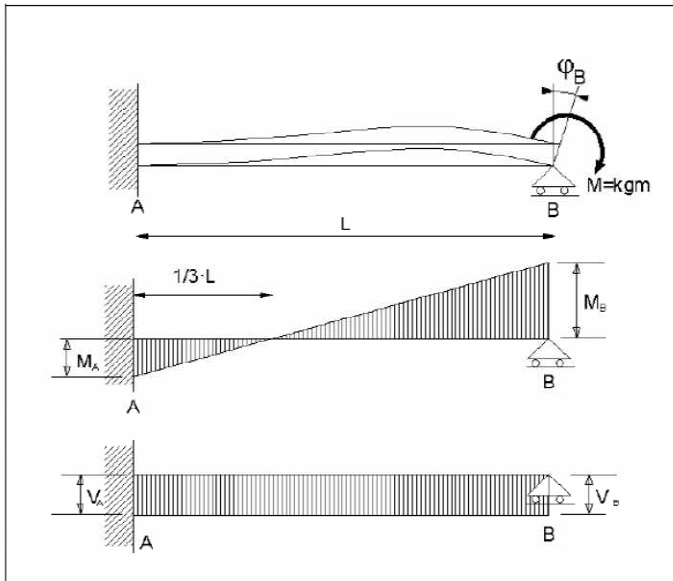
BEAM DEFLECTION FORMULAS

BEAM TYPE	SLOPE AT ENDS	DEFLECTION AT ANY SECTION IN TERMS OF x	MAXIMUM AND CENTER DEFLECTION
6. Beam Simply Supported at Ends – Concentrated load P at the center			
	$\theta_1 = \theta_2 = \frac{Pl^2}{16EI}$	$y = \frac{Px}{12EI}\left(\frac{3l^2}{4} - x^2\right)$ for $0 < x < \frac{l}{2}$	$\delta_{\max} = \frac{Pl^3}{48EI}$
7. Beam Simply Supported at Ends – Concentrated load P at any point			
	$\theta_1 = \frac{Pb(l^2 - b^2)}{6EI}$ $\theta_2 = \frac{Pab(2l - b)}{6EI}$	$y = \frac{Pbx}{6EI}(l^2 - x^2 - b^2)$ for $0 < x < a$ $y = \frac{Pb}{6EI}\left[\frac{l}{b}(x-a)^3 + (l^2 - b^2)x - x^3\right]$ for $a < x < l$	$\delta_{\max} = \frac{Pb(l^2 - b^2)^{3/2}}{9\sqrt{3}EI}$ at $x = \sqrt{(l^2 - b^2)}/3$ $\delta = \frac{Pb}{48EI}(3l^2 - 4b^2)$ at the center, if $a > b$
8. Beam Simply Supported at Ends – Uniformly distributed load ω (N/m)			
	$\theta_1 = \theta_2 = \frac{\omega l^3}{24EI}$	$y = \frac{\omega x}{24EI}(l^3 - 2lx^2 + x^3)$	$\delta_{\max} = \frac{5\omega l^4}{384EI}$
9. Beam Simply Supported at Ends – Couple moment M at the right end			
	$\theta_1 = \frac{Ml}{6EI}$ $\theta_2 = \frac{Ml}{3EI}$	$y = \frac{Mlx}{6EI}\left(1 - \frac{x^2}{l^2}\right)$	$\delta_{\max} = \frac{Ml^2}{9\sqrt{3}EI}$ at $x = \frac{l}{\sqrt{3}}$ $\delta = \frac{Ml^2}{16EI}$ at the center
10. Beam Simply Supported at Ends – Uniformly varying load: Maximum intensity ω_0 (N/m)			
	$\theta_1 = \frac{7\omega_0 l^3}{360EI}$ $\theta_2 = \frac{\omega_0 l^3}{45EI}$	$y = \frac{\omega_0 x}{360EI}(7l^4 - 10l^2x^2 + 3x^4)$	$\delta_{\max} = 0.00652 \frac{\omega_0 l^4}{EI}$ at $x = 0.519l$ $\delta = 0.00651 \frac{\omega_0 l^4}{EI}$ at the center

	<p style="text-align: right;">SCHEMA A1</p> <p>Supported-supported beam with uniform load q</p> $\varphi_A = \varphi_B = \frac{q \cdot L^3}{24 \cdot E \cdot J}$ $f_{\max} = \frac{5}{384} \frac{q \cdot L^4}{E \cdot J}$ $M_{\max} = \frac{q \cdot L^2}{8}$ $V_A = V_B = \frac{q \cdot L}{2}$
	<p style="text-align: right;">SCHEMA A2</p> <p>Supported-supported beam with centered load</p> $\varphi_A = \varphi_B = \frac{P \cdot L^2}{16 \cdot E \cdot J}$ $f_{\max} = \frac{8}{384} \frac{P \cdot L^3}{E \cdot J}$ $M_{\max} = \frac{P \cdot L}{4}$ $V_A = V_B = \frac{P}{2}$

	<p style="text-align: right;">SCHEMA A3</p> <p>Supported-supported beam with moment in B</p> $\varphi_A = \frac{M \cdot L}{6 \cdot E \cdot J} \quad \varphi_B = \frac{M \cdot L}{3 \cdot E \cdot J}$ $f_{L/2} = \frac{1}{16} \cdot \frac{M \cdot L^2}{E \cdot J} \quad f_{MAX} = 1,2064 \cdot f_{L/2}$ $M_A = 0 \quad M_B = M$ $V_A = -V_B = \frac{M}{L}$
	<p style="text-align: right;">SCHEMA B1</p> <p>One end fixed beam with uniform load</p> $\varphi_B = \frac{q \cdot L^3}{6 \cdot E \cdot J}$ $f_{max} = \frac{1}{8} \cdot \frac{q \cdot L^4}{E \cdot J}$ $M_A = \frac{q \cdot L^2}{2}$ $V_A = q \cdot L$
	<p style="text-align: right;">SCHEMA B2</p> <p>One end fixed beam with load in B</p> $\varphi_B = \frac{P \cdot L^2}{2 \cdot E \cdot J}$ $f_B = \frac{1}{3} \cdot \frac{P \cdot L^3}{E \cdot J}$ $M_A = P \cdot L$ $V_A = -V_B = P$
	<p style="text-align: right;">SCHEMA B3</p> <p>One end fixed beam with moment in B</p> $\varphi_B = \frac{M \cdot L}{E \cdot J}$ $f_B = \frac{M \cdot L^2}{2 \cdot E \cdot J}$ $M_A = M \quad V_A = 0$

	<p style="text-align: right;">SCHEMA C1</p> <p>Fixed-supported beam with uniform load q</p> $\varphi_B = \frac{q \cdot L^3}{48 \cdot E \cdot J}$ $f_{L/2} = \frac{2}{384} \cdot \frac{q \cdot L^4}{E \cdot J} \quad f_{\max} = 1,04 \cdot f_{L/2}$ $M_A = \frac{q \cdot L^2}{8} \quad M_{5/8 \cdot L} = \frac{q \cdot L^2}{14,22}$ $V_A = \frac{5}{8} \cdot q \cdot L \quad V_B = \frac{3}{8} \cdot q \cdot L$
	<p style="text-align: right;">SCHEMA C2</p> <p>Fixed-supported beam with concentrated load in the center</p> $\varphi_B = \frac{P \cdot L^2}{32 \cdot E \cdot J}$ $f_{L/2} = \frac{3,5}{384} \cdot \frac{P \cdot L^3}{E \cdot J} \quad f_{\max} = 1,022 \cdot f_{L/2}$ $M_A = \frac{3}{16} \cdot P \cdot L \quad M_{L/2} = \frac{2,5}{16} \cdot P \cdot L$ $V_A = \frac{11}{16} \cdot P \quad V_B = \frac{5}{16} \cdot P$



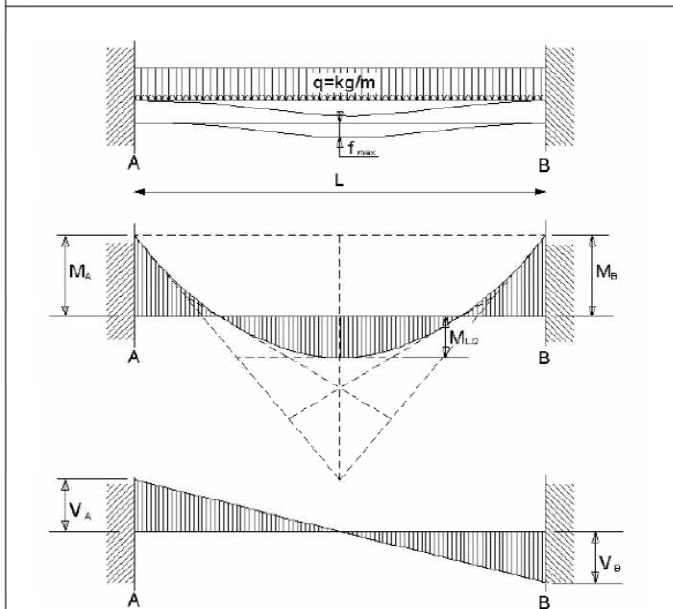
SCHEMA C3

Fixed-supported beam with moment M in B

$$\varphi_B = \frac{M \cdot L}{4 \cdot E \cdot J}$$

$$M_A = \frac{M}{2} \quad M_B = M$$

$$V_A = -V_B = \frac{3 \cdot M}{2 \cdot L}$$



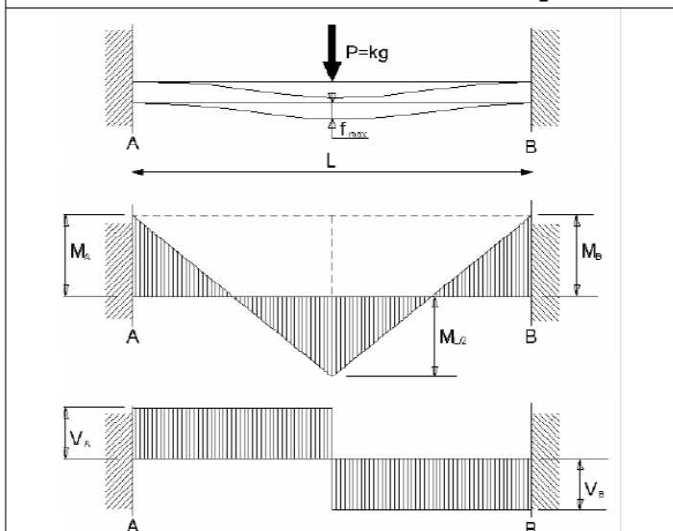
SCHEMA D1

Fixed-fixed beam with uniform load

$$f_{MAX} = \frac{1}{384} \frac{q \cdot L^4}{E \cdot J}$$

$$M_A = M_B = \frac{q \cdot L^2}{12} \quad M_{L/2} = \frac{q \cdot L^2}{24}$$

$$V_A = V_B = \frac{q \cdot L}{2}$$



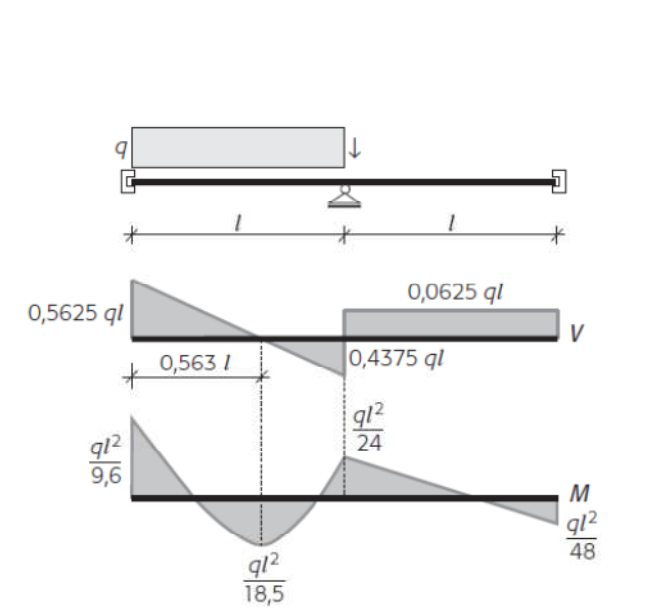
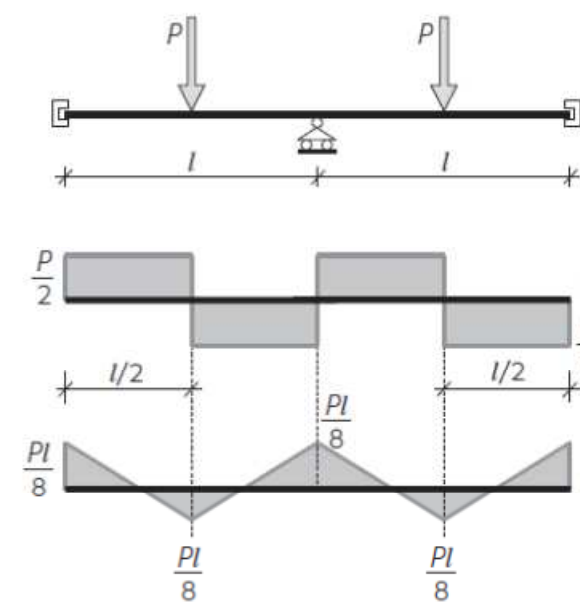
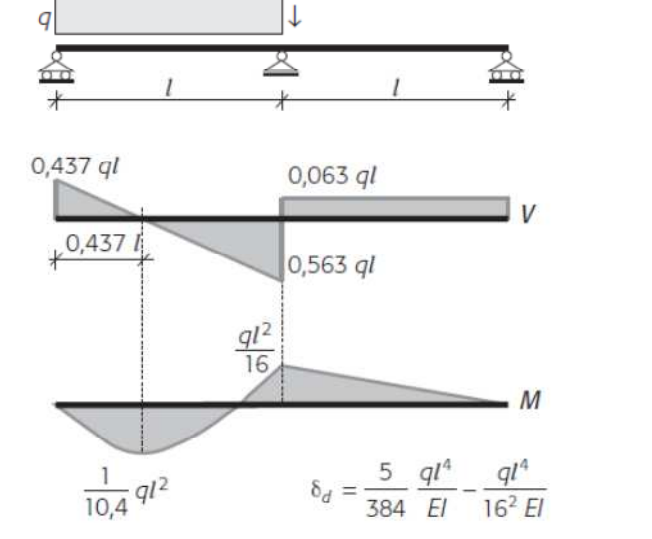
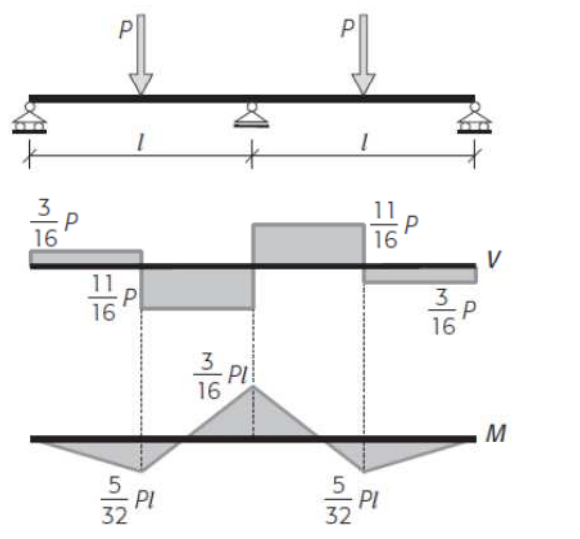
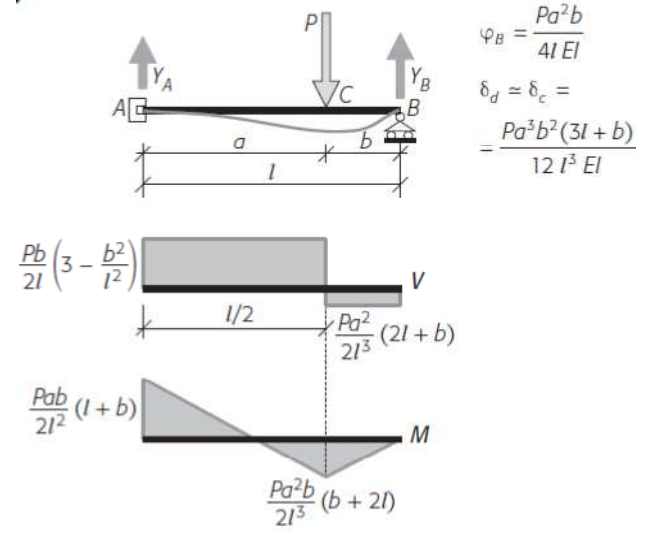
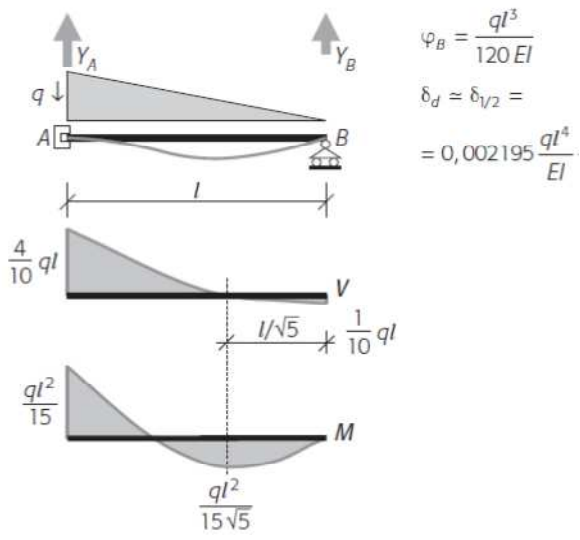
SCHEMA D2

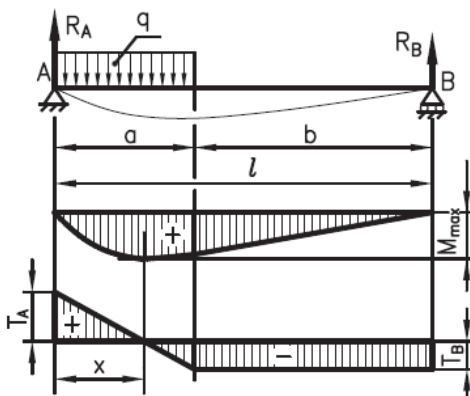
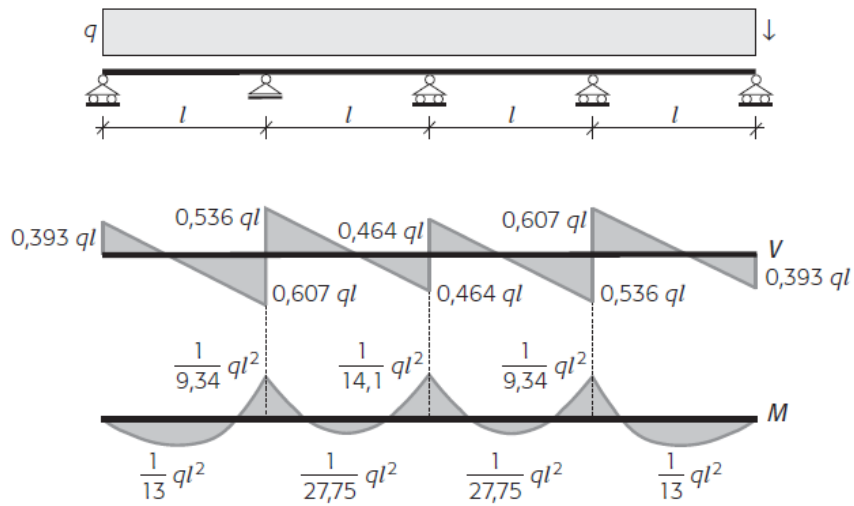
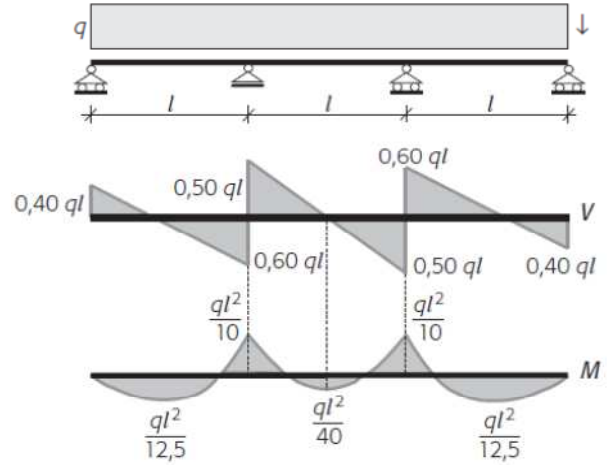
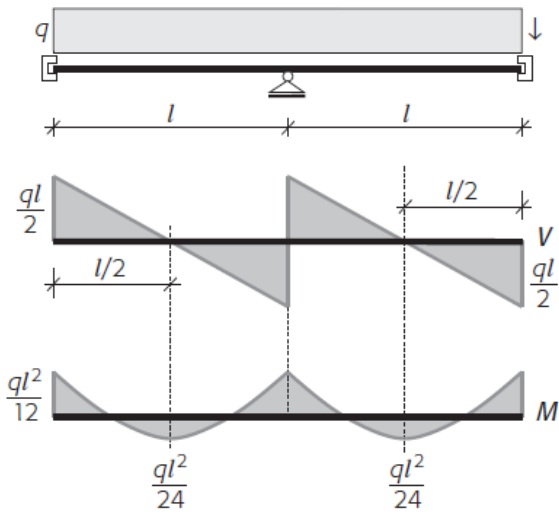
Fixed-fixed beam with concentrated load in the center

$$f_{Max} = \frac{2}{384} \frac{P \cdot L^3}{E \cdot J}$$

$$M_A = M_B = \frac{P \cdot L}{8} \quad M_{L/2} = \frac{P \cdot L}{8}$$

$$V_A = V_B = \frac{P}{2}$$





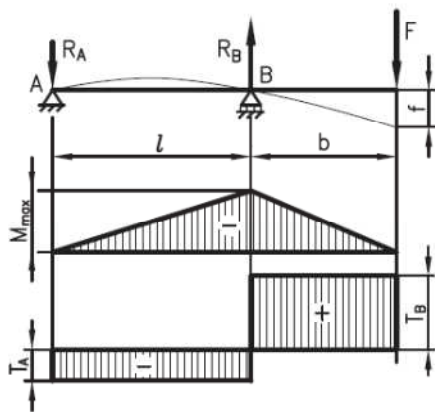
$$R_A = q \cdot a \cdot \frac{a + 2b}{2l}; \quad R_B = q \cdot \frac{a^2}{2l}$$

$$T_A = R_A; \quad T_B = -R_B$$

$$M_A = M_B = 0$$

$$M_{max} = \frac{R_A^2}{q}$$

$$x = \frac{R_A}{q}$$



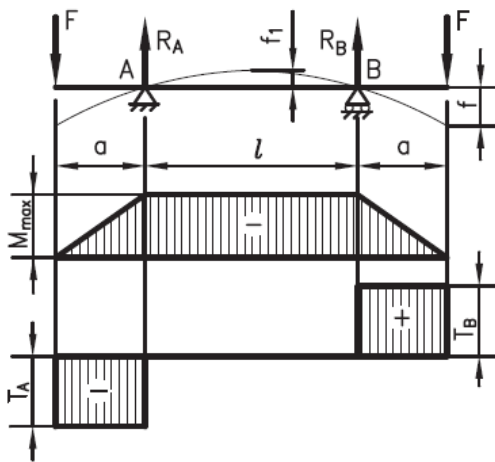
$$R_A = \frac{b}{l} \cdot F; \quad R_B = \frac{l+a}{l} \cdot F$$

$$T_A = -\frac{b}{l} \cdot F; \quad T_B = F$$

$$M_A = 0$$

$$M_{max} = -F \cdot b$$

$$f = \frac{F}{E \cdot I} \cdot \frac{(l+b) \cdot b^2}{3}$$



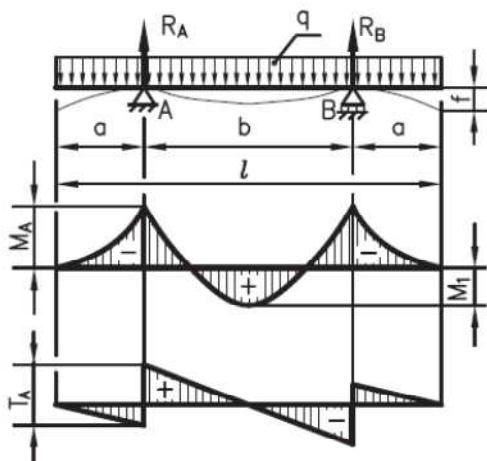
$$R_A = R_B = F$$

$$T_A = -R_A; \quad T_B = R_B$$

$$M_{max} = -F \cdot a$$

$$f_1 = \frac{F \cdot a \cdot l^2}{8 \cdot E \cdot I}$$

$$f = \frac{F \cdot a^2}{3 \cdot E \cdot I} \cdot \left(a + \frac{3l}{2} \right)$$



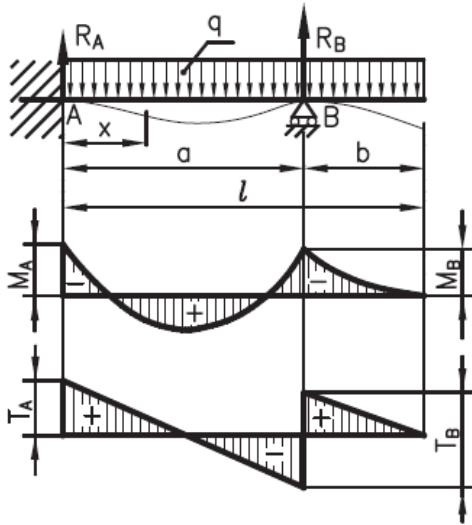
$$R_A = R_B = \frac{q \cdot l^2}{2}$$

$$T_A = |R_A|; \quad T_B = |R_B|$$

$$M_A = \frac{-q \cdot a}{2}$$

$$M_1 = \frac{q \cdot l^2}{4} \cdot \left(\frac{b}{l} - \frac{1}{2} \right)$$

$$f = \frac{q \cdot a}{24 \cdot E \cdot I} \cdot (3a^2 - b^3 + 6a^2 \cdot b)$$



$$R_A = \frac{q \cdot l}{2} \cdot \left(3 - \frac{3l}{2a} - \frac{a}{4l} \right)$$

$$R_B = \frac{q \cdot l}{2} \cdot \left(\frac{3l}{2a} + \frac{a}{4l} - 1 \right)$$

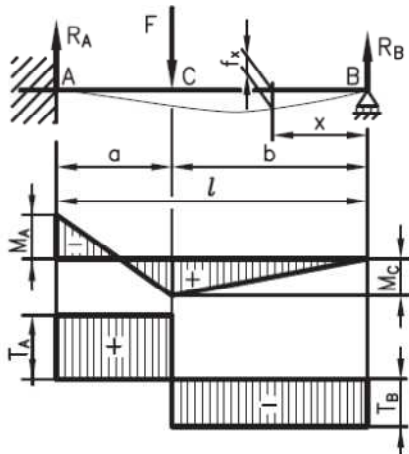
$$T_A = R_A; \quad T_B = |R_B|$$

$$M_A = q \cdot l \cdot \frac{2b^2 - a^2}{8l}$$

$$M_B = -\frac{q \cdot b^2}{2}$$

$$M_A = M_{max} \quad \text{se } a > \sqrt{6} \cdot b$$

$$M_B = M_{max} \quad \text{se } a < \sqrt{6} \cdot b$$



$$R_A = \frac{F}{2l^3} \cdot (3l^2 - b^2) \cdot b$$

$$R_B = \frac{F}{2l^3} \cdot (2l + b) \cdot a^2$$

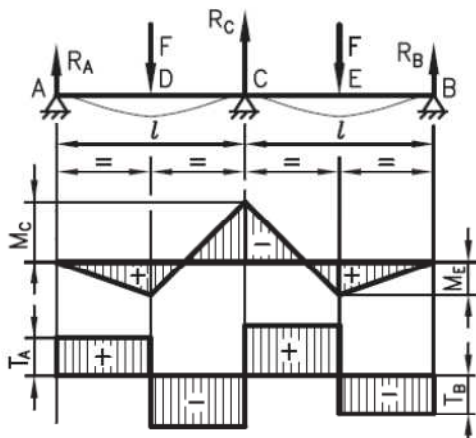
$$T_A = R_A; \quad T_B = -R_B$$

$$M_A = -\frac{F \cdot a \cdot b \cdot (l + b)}{2l^2}$$

$$M_C = \frac{F \cdot a^2 \cdot b \cdot (2l + b)}{2l^3}$$

$$f_C = \frac{F}{E \cdot I} \cdot \frac{a^3 \cdot b^2 \cdot (3l + b)}{12l^3}$$

$$f_x = \frac{F \cdot a^3}{12E \cdot I} \left[3b - (2l + b) \cdot \left(\frac{l-x}{l} \right) \right] \left(\frac{l-x}{l} \right)$$

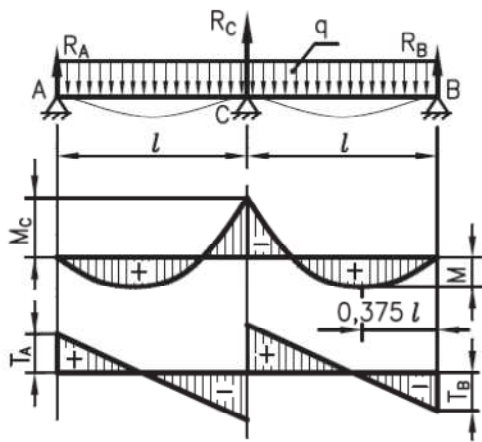


$$R_A = R_B = \frac{5}{16} \cdot F; \quad R_C = \frac{22}{16} \cdot F$$

$$T_A = R_A; \quad T_B = -R_B$$

$$M_A = M_B = \frac{5}{32} \cdot F \cdot l$$

$$M_C = -\frac{3}{16} \cdot F \cdot l$$

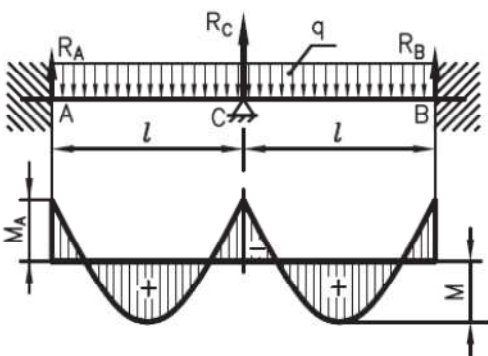


$$R_A = R_B = \frac{3}{8} \cdot q \cdot l; \quad R_C = \frac{5}{4} \cdot q \cdot l$$

$$T_A = R_A; \quad T_B = -R_B$$

$$M = \frac{9}{128} \cdot q \cdot l^2$$

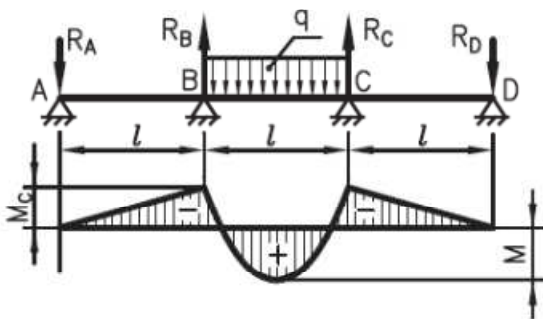
$$M_C = -\frac{1}{8} \cdot q \cdot l^2$$



$$R_A = R_B = \frac{1}{2} \cdot q \cdot l; \quad R_C = q \cdot l$$

$$M = \frac{1}{24} \cdot q \cdot l^2$$

$$M_A = M_B = M_C = -\frac{1}{12} \cdot q \cdot l^2$$

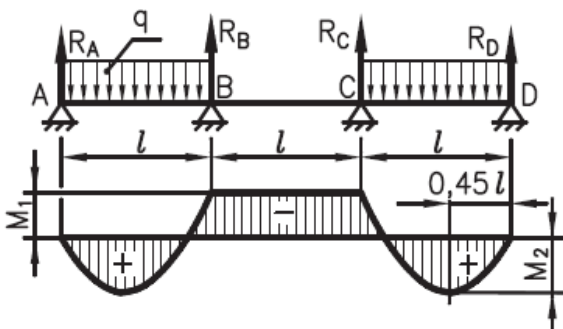


$$R_A = R_D = 0,005 \cdot q \cdot l$$

$$R_B = R_C = 0,550 \cdot q \cdot l$$

$$M = \frac{5}{67} \cdot q \cdot l^2$$

$$M_A = M_B = M_C = -\frac{1}{20} \cdot q \cdot l^2$$

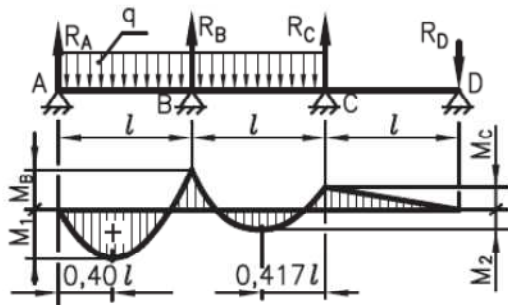


$$R_A = R_D = 0,450 \cdot q \cdot l$$

$$R_B = R_C = 0,550 \cdot q \cdot l$$

$$M_1 = -\frac{1}{20} \cdot q \cdot l^2$$

$$M_2 = \frac{10}{99} \cdot q \cdot l^2$$

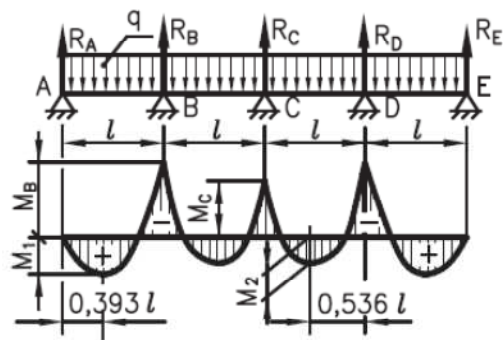


$$R_A = 0,383 \cdot q \cdot l; \quad R_B = 1,2 \cdot q \cdot l$$

$$R_C = 0,450 \cdot q \cdot l; \quad R_D = 0,033 \cdot q \cdot l$$

$$M_1 = \frac{1}{12,7} \cdot q \cdot l^2; \quad M_B = -\frac{1}{8,55} \cdot q \cdot l^2$$

$$M_C = -\frac{1}{30,3} \cdot q \cdot l^2; \quad M_2 = \frac{1}{18,3} \cdot q \cdot l^2$$



$$R_A = 0,393 \cdot q \cdot l \quad R_B = 1,143 \cdot q \cdot l$$

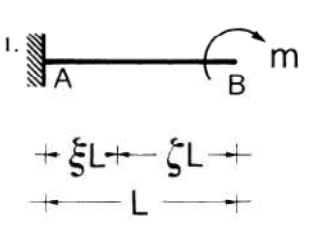
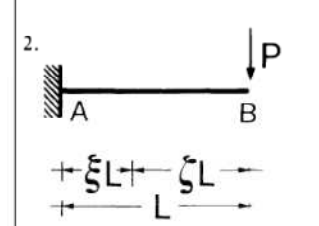
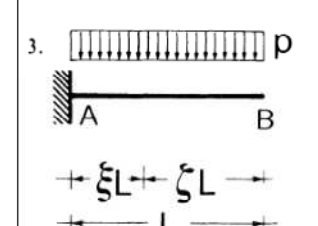
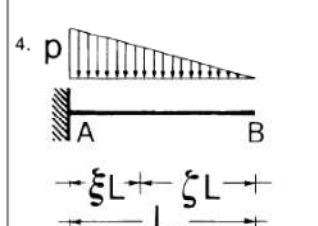
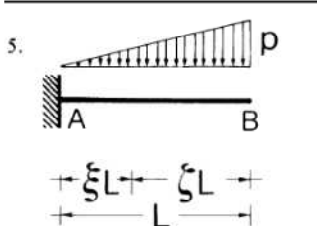
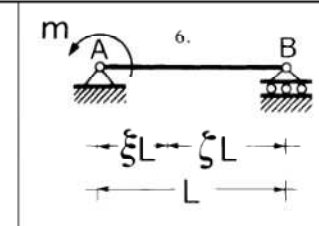
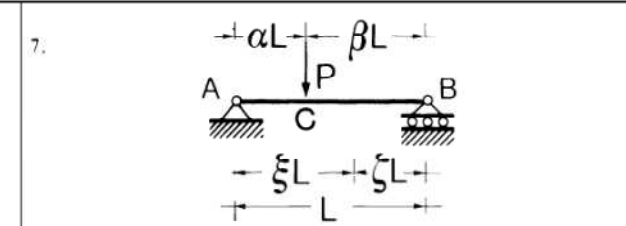
$$R_C = 0,929 \cdot q \cdot l; \quad R_D = 1,143 \cdot q \cdot l$$

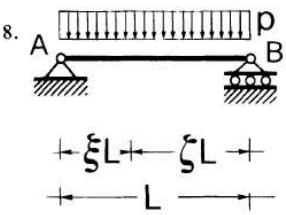
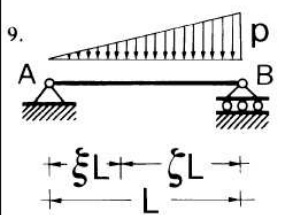
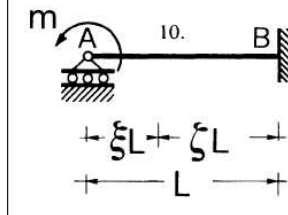
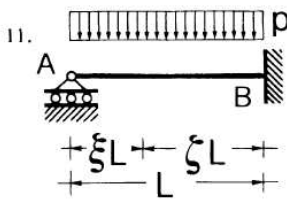
$$R_E = 0,393 \cdot q \cdot l$$

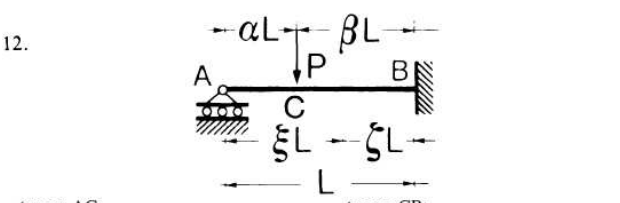
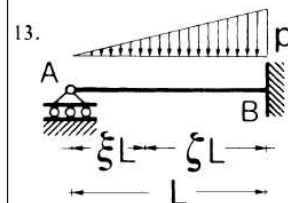
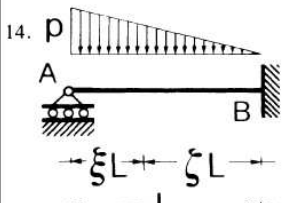
$$M_1 = \frac{1}{13} \cdot q \cdot l^2; \quad M_B = -\frac{1}{8,55} \cdot q \cdot l^2$$

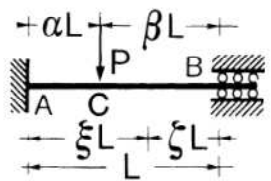
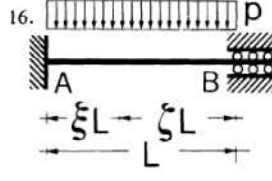
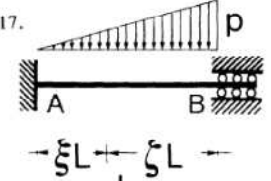
$$M_C = -\frac{1}{14,1} \cdot q \cdot l^2; \quad M_2 = \frac{1}{27,75} \cdot q \cdot l^2$$

Tabella 6. Travi ad asse rettilineo, dati per varie situazioni di carico e vincolo

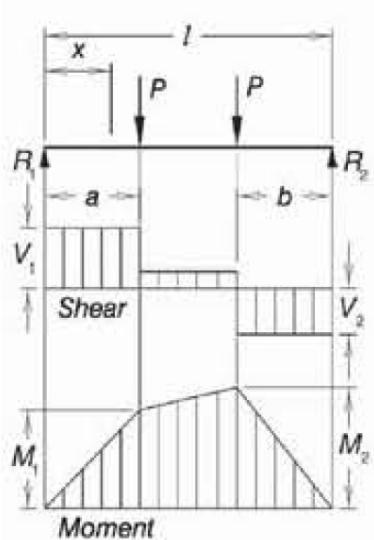
<p>1. </p> <p>$Y_A = 0; M_A = -m$</p> <p>$T = cost = 0$</p> <p>$M = cost = -m$</p> <p>$\varphi = \frac{mL}{EI} \xi$</p> <p>$\varphi_B = \frac{mL}{EI}$</p> <p>$v = \frac{1}{2} \frac{mL^2}{EI} \xi^2$</p> <p>$v_B = \frac{1}{2} \frac{mL^2}{EI}$</p>	<p>2. </p> <p>$Y_A = P; M_A = -PL$</p> <p>$T = cost = P$</p> <p>$M = -PL \zeta$</p> <p>$\varphi = \frac{1}{2} \frac{PL^2}{EI} \xi (1 + \zeta)$</p> <p>$\varphi_B = \frac{1}{2} \frac{PL^2}{EI}$</p> <p>$v = \frac{1}{6} \frac{PL^3}{EI} \xi^2 (3 - \zeta)$</p> <p>$v_B = \frac{1}{3} \frac{PL^3}{EI}$</p>	<p>3. </p> <p>$Y_A = PL; M_A = -\frac{1}{2} PL^2$</p> <p>$T = pL \zeta$</p> <p>$M = -\frac{1}{2} pL^2 \zeta^2$</p> <p>$\varphi = \frac{1}{6} \frac{pL^3}{EI} (1 - \zeta^3)$</p> <p>$\varphi_B = \frac{1}{6} \frac{pL^3}{EI}$</p> <p>$v = \frac{1}{24} \frac{pL^4}{EI} [3 - \zeta (4 - \zeta^3)]$</p> <p>$v_B = \frac{1}{8} \frac{pL^4}{EI}$</p>	<p>4. </p> <p>$Y_A = \frac{1}{2} PL; M_A = -\frac{1}{6} PL^2$</p> <p>$T = \frac{1}{2} pL \zeta^2$</p> <p>$M = -\frac{1}{6} pL^2 \zeta^3$</p> <p>$\varphi = \frac{1}{24} \frac{pL^3}{EI} (1 - \zeta^4)$</p> <p>$\varphi_B = \frac{1}{24} \frac{pL^3}{EI}$</p> <p>$v = \frac{1}{120} \frac{pL^4}{EI} [4 - \zeta (5 - \zeta^4)]$</p> <p>$v_B = \frac{1}{30} \frac{pL^4}{EI}$</p>
<p>5. </p> <p>$Y_A = \frac{1}{2} pL; M_A = -\frac{1}{3} pL^2$</p> <p>$T = \frac{1}{2} pL \zeta (2 - \zeta)$</p> <p>$M = -\frac{1}{6} pL^2 (3 - \zeta) \zeta^2$</p> <p>$\varphi = \frac{1}{24} \frac{pL^3}{EI} [3 - \zeta^3 (4 - \zeta)]$</p> <p>$\varphi_B = \frac{1}{8} \frac{pL^3}{EI}$</p> <p>$v = \frac{1}{120} \frac{pL^4}{EI} [11 - 15 \zeta + \zeta^4 (5 - \zeta)]$</p> <p>$v_B = \frac{11}{120} \frac{pL^4}{EI}$</p>	<p>6. </p> <p>$Y_A = -Y_B = \frac{m}{L}$</p> <p>$T = cost = \frac{m}{L}$</p> <p>$M = -m \zeta$</p> <p>$\varphi = \frac{1}{6} \frac{mL}{EI} (1 - 3 \zeta^2)$</p> <p>$\varphi_A = -\frac{1}{3} \frac{mL}{EI}$</p> <p>$\varphi_B = \frac{1}{6} \frac{mL}{EI}$</p> <p>$v = -\frac{1}{6} \frac{mL^2}{EI} \zeta (1 - \zeta^2)$</p>	<p>7. </p> <p>– tronco AC:</p> <p>$Y_A = P \beta$</p> <p>$T = cost = P\beta$</p> <p>$M = PL \beta \xi$</p> <p>$\varphi = \frac{1}{6} \frac{PL^2}{EI} \beta (1 - \beta^2 - 3 \xi^2)$</p> <p>$\varphi_A = \frac{1}{6} \frac{PL^2}{EI} \beta (1 - \beta^2)$</p> <p>$\varphi_C = \frac{1}{3} \frac{PL^2}{EI} \alpha \beta (\beta - \alpha)$</p> <p>$v_A = \frac{1}{6} \frac{PL^3}{EI} \beta \xi (1 - \beta^2 - \xi^2)$</p> <p>$v_C = \frac{1}{3} \frac{PL^3}{EI} \alpha^2 \beta^2$</p> <p>– tronco CB:</p> <p>$Y_B = P \alpha$</p> <p>$T = cost = -P \alpha$</p> <p>$M = PL \alpha \zeta$</p> <p>$M_{max} = M_C = PL \alpha \beta$</p> <p>$\varphi = -\frac{1}{6} \frac{PL^2}{EI} \alpha (1 - \alpha^2 - 3 \zeta^2)$</p> <p>$\varphi_B = -\frac{1}{6} \frac{PL^2}{EI} \alpha (1 - \alpha^2)$</p> <p>$v_B = \frac{1}{6} \frac{PL^3}{EI} \alpha \zeta (1 - \alpha^2 - \zeta^2)$</p>	

<p>8. </p> $Y_A = Y_B = \frac{1}{2} pL$ $T = \frac{1}{2} pL (1 - 2 \xi)$ $M = \frac{1}{2} pL^2 \zeta \xi$ $\xi = \frac{1}{2} : M = M_{\max} = \frac{1}{8} pL^2$ $\varphi = \frac{1}{24} \frac{pL^2}{EI} [1 + 2 \xi^2 (2 \xi - 3)]$ $\varphi_A = -\varphi_B = \frac{1}{24} \frac{pL^3}{EI}$ $v = \frac{1}{24} \frac{pL^4}{EI} \zeta \xi (1 + \zeta \xi)$ $\xi = \frac{1}{2} : v = v_{\max} = \frac{5}{384} \frac{pL^4}{EI}$	<p>9. </p> $Y_A = \frac{1}{6} pL; \quad Y_B = \frac{1}{3} pL$ $T = \frac{1}{6} pL (1 - 3 \xi^2)$ $M = \frac{1}{6} pL^2 \zeta \xi (1 + \xi)$ $\xi = \frac{\sqrt{3}}{3} : M = M_{\max} = \frac{\sqrt{3}}{27} pL^2$ $\varphi = \frac{1}{360} \frac{pL^2}{EI} [7 - 15 \xi^2 (2 - \xi^2)]$ $\varphi_A = \frac{7}{360} \frac{pL^3}{EI}$ $\varphi_B = -\frac{8}{360} \frac{pL^3}{EI}$ $v = \frac{1}{360} \frac{pL^4}{EI} \zeta \xi (1 + \xi) (7 - 3 \xi^2)$	<p>10. </p> $Y_A = -Y_B = \frac{3}{2} \frac{m}{L}$ $M_B = \frac{1}{2} m$ $T = \text{const} = \frac{3}{2} \frac{m}{L}$ $M = -\frac{1}{2} m (2 - 3 \xi)$ $\varphi = \frac{1}{4} \frac{mL}{EI} \zeta (2 - 3 \xi)$ $\varphi_A = -\frac{1}{4} \frac{mL}{EI}$ $v = -\frac{1}{4} \frac{mL^2}{EI} \xi \zeta^2$ $\xi = \frac{1}{3} : v = v_{\min} = -\frac{1}{27} \frac{mL^2}{EI}$	<p>11. </p> $Y_A = \frac{3}{8} pL; \quad Y_B = \frac{5}{8} pL$ $M_B = -\frac{1}{8} pL^2$ $T = pL \left(\frac{3}{8} - \xi \right)$ $M = \frac{1}{8} pL^2 \xi (3 - 4 \xi)$ $\varphi = \frac{1}{48} \frac{pL^3}{EI} \zeta (1 + \xi - 8 \xi^2)$ $\varphi_A = \frac{1}{48} \frac{pL^3}{EI}$ $v = \frac{1}{48} \frac{pL^4}{EI} \xi \zeta^2 (1 + 2 \xi)$
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<p>12. </p> <p>- tronco AC: $Y_A = \frac{1}{2} P \beta^2 (2 + \alpha)$</p> <p>$T = \text{const} = Y_A$</p> <p>$M = \frac{1}{2} PL \xi \beta^2 (2 + \alpha)$</p> <p>$\varphi = \frac{1}{4} \frac{PL^2}{EI} \beta^2 [\alpha - (2 + \alpha) \xi^2];$</p> <p>$\varphi_A = \frac{1}{4} \frac{PL^2}{EI} \alpha \beta^2$</p> <p>$\varphi_C = \frac{1}{4} \frac{PL^2}{EI} \alpha \beta^2 (1 - 2\alpha - \alpha^2)$</p> <p>$v = \frac{1}{12} \frac{PL^3}{EI} \xi \beta^2 [3\alpha - (2 + \alpha) \xi^2]$</p> <p>$v_C = \frac{1}{12} \frac{PL^3}{EI} \alpha^2 \beta^3 (3 + \alpha)$</p> <p>- tronco CB: $Y_B = \frac{1}{2} P \alpha (3 - \alpha^2)$</p> <p>$M_B = -\frac{1}{2} PL \alpha (1 - \alpha^2)$</p> <p>$T = \text{const} = -Y_B$</p> <p>$M = \frac{1}{2} PL \alpha (3 \zeta + \alpha^2 \zeta - 1)$</p> <p>$\varphi = \frac{1}{4} \frac{PL^2}{EI} \alpha \zeta [\alpha^2 (2 - \zeta) + 3\zeta - 2]$</p> <p>$v = \frac{1}{12} \frac{PL^3}{EI} \alpha \zeta^2 [3(1 - \alpha^2) + (3 - \alpha^2)\zeta]$</p>	<p>13. </p> $Y_A = \frac{1}{10} pL; \quad Y_B = \frac{2}{5} pL$ $M_B = -\frac{1}{15} pL^2$ $T = \frac{1}{10} pL (1 - 5 \xi^2)$ $M = \frac{1}{10} pL^2 \xi (3 - 5 \xi^2)$ $\varphi = \frac{1}{120} \frac{pL^3}{EI} \cdot \zeta (1 + \xi) (1 - 5 \xi^2)$ $\varphi_A = \frac{1}{120} \frac{pL^3}{EI}$ $v = \frac{1}{120} \frac{pL^4}{EI} \xi \zeta^2 (1 + \xi)^2$	<p>14. </p> $Y_A = \frac{11}{40} pL; \quad Y_B = \frac{9}{40} pL$ $M_B = -\frac{7}{120} pL^2$ $T = \frac{1}{40} pL (20 \zeta^2 - 9)$ $M = \frac{1}{120} pL^2 \xi (20 \zeta^2 + 20 \zeta - 7)$ $\varphi = \frac{1}{240} \frac{pL^3}{EI} \zeta (27 \zeta + 10 \zeta^3 - 14)$ $\varphi_A = \frac{1}{80} \frac{pL^3}{EI}$ $v = \frac{1}{240} \frac{pL^4}{EI} \xi \zeta^2 (7 - 2 \zeta - 2 \zeta^2)$
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<p>15. </p> <p>– tronco AC: – tronco CB:</p> <p>$Y_A = P \beta^2 (1 + 2 \alpha)$ $Y_B = P \alpha^2 (1 + 2 \beta)$</p> <p>$M_A = - PL \alpha \beta^2$ $M_B = - PL \alpha^2 \beta$</p> <p>$T = cost = Y_A$ $T = cost = - Y_B$</p> <p>$M = PL \beta^2 [\xi (1 + 2 \alpha) - \alpha]$ $M = PL \alpha^2 [\zeta (1 + 2 \beta) - \beta]$</p> <p style="text-align: center;">$M_C = 2 PL \alpha^2 \beta^2$</p> <p>$\varphi = \frac{1}{2} \frac{PL^2}{EI} \beta^2 \xi [2\alpha - \xi (1 + 2\alpha)]$ $\varphi = -\frac{1}{2} \frac{PL^2}{EI} \alpha^2 \zeta [2\beta - \zeta (1 + 2\beta)]$</p> <p>$\varphi_C = \frac{1}{2} \frac{PL^2}{EI} \alpha^2 \beta^2 (1 - 2 \alpha)$</p> <p>$v = \frac{1}{6} \frac{PL^3}{EI} \beta^2 \xi^2 [3\alpha - \xi (1 + 2\alpha)]$ $v = \frac{1}{6} \frac{PL^3}{EI} \alpha^2 \zeta^2 [3\beta - \zeta (1 + 2\beta)]$</p> <p>$v_C = \frac{1}{3} \frac{PL^3}{EI} \alpha^3 \beta^3$</p>	<p>16. </p> <p>$Y_A = Y_B = \frac{1}{2} pL$</p> <p>$M_A = M_B = -\frac{1}{12} pL^2$</p> <p>$T = \frac{1}{2} pL (1 - 2 \xi)$</p> <p>$M = -\frac{1}{12} pL^2 (1 - 6 \xi \zeta)$</p> <p>$\xi = \frac{1}{2} : M = \frac{1}{24} pL^2$</p> <p>$\varphi = \frac{1}{12} \frac{pL^2}{EI} \xi \zeta (1 - 2 \xi)$</p> <p>$v = \frac{1}{24} \frac{pL^4}{EI} \xi^2 \zeta^2$</p> <p>$\xi = \frac{1}{2} : v = \frac{1}{384} \frac{pL^4}{EI}$</p>	<p>17. </p> <p>$Y_A = \frac{3}{20} pL : Y_B = \frac{7}{20} pL$</p> <p>$M_A = -\frac{1}{30} pL^2 ; M_B = \frac{1}{20} pL^2$</p> <p>$T = \frac{1}{20} pL (3 - 10 \xi^2)$</p> <p>$M = -\frac{1}{60} pL^2 (2 - 9 \xi + 10 \xi^3)$</p> <p>$\varphi = \frac{1}{120} \frac{pL^3}{EI} \xi \zeta (4 - 5 \xi - 5 \xi^2)$</p> <p>$v = \frac{1}{120} \frac{pL^4}{EI} \xi^2 \zeta^2 (2 + \xi)$</p>
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10. SIMPLE BEAM — TWO EQUAL CONCENTRATED LOADS UNSYMMETRICALLY PLACED



$R_1 = V_1 (= V_{max} \text{ when } a < b) \dots\dots\dots = \frac{P}{l}(l - a + b)$

$R_2 = V_2 (= V_{max} \text{ when } a > b) \dots\dots\dots = \frac{P}{l}(l - b + a)$

$V_x \text{ (when } a < x < (l - b)) \dots\dots\dots = \frac{P}{l}(b - a)$

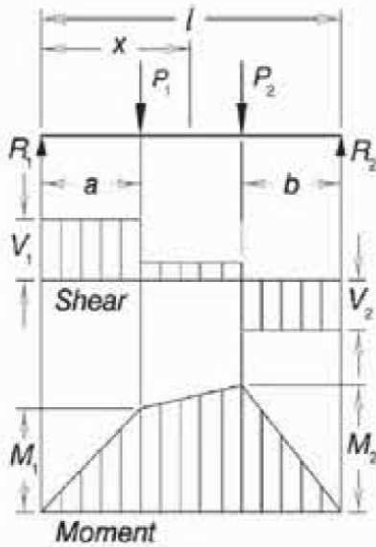
$M_1 \text{ (= } M_{max} \text{ when } a > b) \dots\dots\dots = R_1 a$

$M_2 \text{ (= } M_{max} \text{ when } a < b) \dots\dots\dots = R_2 b$

$M_x \text{ (when } x < a) \dots\dots\dots = R_1 x$

$M_x \text{ (when } a < x < (l - b)) \dots\dots\dots = R_1 x - P(x - a)$

11. SIMPLE BEAM — TWO UNEQUAL CONCENTRATED LOADS UNSYMMETRICALLY PLACED



$$R_1 = V_1 \dots\dots\dots = \frac{P_1(l-a) + P_2 b}{l}$$

$$R_2 = V_2 \dots\dots\dots = \frac{P_1 a + P_2(l-b)}{l}$$

$$V_x \text{ (when } a < x < (l-b)) \dots\dots\dots = R_1 - P_1$$

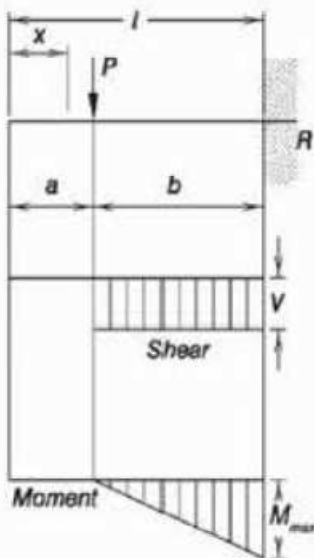
$$M_1 \text{ (= } M_{max} \text{ when } R_1 < P_1) \dots\dots\dots = R_1 a$$

$$M_2 \text{ (= } M_{max} \text{ when } R_2 < P_2) \dots\dots\dots = R_2 b$$

$$M_x \text{ (when } x < a) \dots\dots\dots = R_1 x$$

$$M_x \text{ (when } a < x < (l-b)) \dots\dots\dots = R_1 x - P_1(x-a)$$

21. CANTILEVERED BEAM — CONCENTRATED LOAD AT ANY POINT



$$\text{Total Equiv. Uniform Load} \dots\dots\dots = \frac{8Pb}{l}$$

$$R = V \dots\dots\dots = P$$

$$M_{max} \text{ (at fixed end)} \dots\dots\dots = Pb$$

$$M_x \text{ (when } x > a) \dots\dots\dots = P(x-a)$$

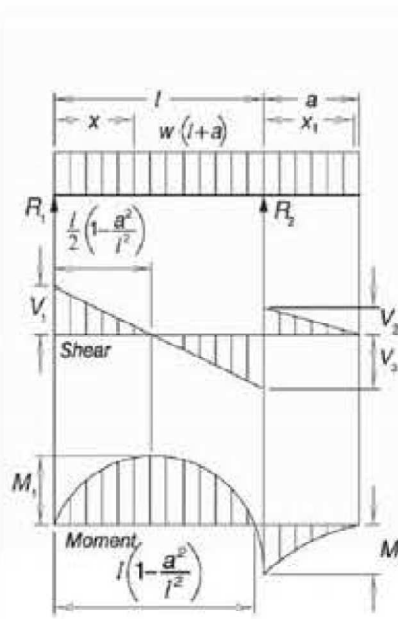
$$\Delta_{max} \text{ (at free end)} \dots\dots\dots = \frac{Pb^2}{6EI} (3l-b)$$

$$\Delta_a \text{ (at point of load)} \dots\dots\dots = \frac{Pb^3}{3EI}$$

$$\Delta_x \text{ (when } x < a) \dots\dots\dots = \frac{Pb^2}{6EI} (3l-3x-b)$$

$$\Delta_x \text{ (when } x > a) \dots\dots\dots = \frac{P(l-x)^2}{6EI} (3b-l+x)$$

24. BEAM OVERHANGING ONE SUPPORT — UNIFORMLY DISTRIBUTED LOAD



$$R_1 = V_1 \dots\dots\dots = \frac{w}{2l}(l^2 - a^2)$$

$$R_2 = V_2 + V_3 \dots\dots\dots = \frac{w}{2l}(l+a)^2$$

$$V_2 \dots\dots\dots = wa$$

$$V_3 \dots\dots\dots = \frac{w}{2l}(l^2 + a^2)$$

$$V_x \text{ (between supports) } \dots\dots\dots = R_1 - wx$$

$$V_{x_1} \text{ (for overhang) } \dots\dots\dots = w(a - x_1)$$

$$M_x \left(\text{at } x = \frac{l}{2} \left[1 - \frac{a^2}{l^2} \right] \right) \dots\dots\dots = \frac{w}{8l^2}(l+a)^2(l-a)^2$$

$$M_2 \text{ (at } R_2) \dots\dots\dots = \frac{wa^2}{2}$$

$$M_x \text{ (between supports) } \dots\dots\dots = \frac{wx}{2l}(l^2 - a^2 - xl)$$

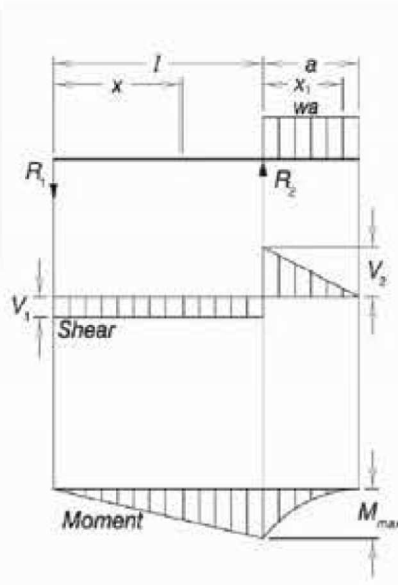
$$M_{x_1} \text{ (for overhang) } \dots\dots\dots = \frac{w}{2}(a - x_1)^2$$

$$\Delta_x \text{ (between supports) } \dots\dots\dots = \frac{wx}{24EI} (l^4 - 2l^2x^2 + lx^3 - 2a^2l^2 + 2a^2x^2)$$

$$\Delta_{x_1} \text{ (for overhang) } \dots\dots\dots = \frac{wx_1}{24EI} (4a^2l - l^3 + 6a^2x_1 - 4ax_1^2 + x_1^3)$$

NOTE: For a negative value of Δ_x , deflection is upward.

25. BEAM OVERHANGING ONE SUPPORT — UNIFORMLY DISTRIBUTED LOAD ON OVERHANG



$$R_1 = V_1 \dots\dots\dots = \frac{wa^2}{2l}$$

$$R_2 = V_1 + V_2 \dots\dots\dots = \frac{wa}{2l}(2l+a)$$

$$V_2 \dots\dots\dots = wa$$

$$V_{x_1} \text{ (for overhang) } \dots\dots\dots = w(a - x_1)$$

$$M_{max} \text{ (at } R_2) \dots\dots\dots = \frac{wa^2}{2}$$

$$M_x \text{ (between supports) } \dots\dots\dots = \frac{wa^2x}{2l}$$

$$M_{x_1} \text{ (for overhang) } \dots\dots\dots = \frac{w}{2}(a - x_1)^2$$

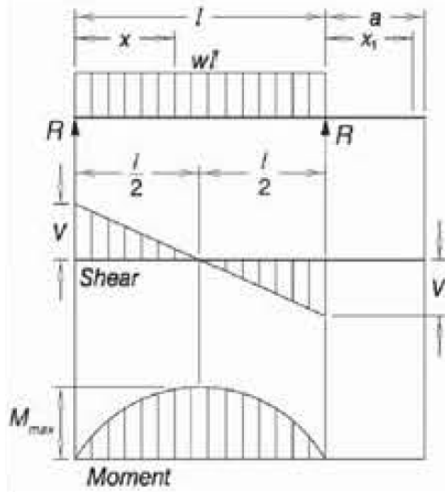
$$\Delta_{max} \left(\text{between supports at } x = \frac{l}{\sqrt{3}} \right) = \frac{wa^2l^2}{18\sqrt{3}EI} = 0.0321 \frac{wa^2l^2}{EI}$$

$$\Delta_{max} \text{ (for overhang at } x_1 = a) \dots\dots\dots = \frac{wa^3}{24EI} (4l+3a)$$

$$\Delta_x \text{ (between supports) } \dots\dots\dots = \frac{wa^2x}{12EI} (l^2 - x^2)$$

$$\Delta_{x_1} \text{ (for overhang) } \dots\dots\dots = \frac{wx_1}{24EI} (4a^2l + 6a^2x_1 - 4ax_1^2 + x_1^3)$$

27. BEAM OVERHANGING ONE SUPPORT — UNIFORMLY DISTRIBUTED LOAD BETWEEN SUPPORTS



Total Equiv. Uniform Load = wl

$R = V$ = $\frac{wl}{2}$

V_x = $w\left(\frac{l}{2} - x\right)$

M_{max} (at center) = $\frac{wl^2}{8}$

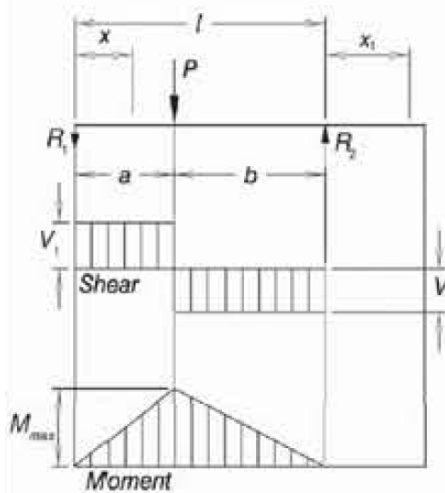
M_x = $\frac{wx}{2}(l - x)$

$\Delta_{y_{max}}$ (at center) = $\frac{5wl^4}{384EI}$

Δ_x = $\frac{wx}{24EI}(l^3 - 2lx^2 + x^3)$

Δ_{x_1} = $\frac{wl^3 x_1}{24EI}$

28. BEAM OVERHANGING ONE SUPPORT — CONCENTRATED LOAD AT ANY POINT BETWEEN SUPPORTS



Total Equiv. Uniform Load = $\frac{8Pab}{l^2}$

$R_1 = V_1 (= V_{max} \text{ when } a < b)$ = $\frac{Pb}{l}$

$R_2 = V_2 (= V_{max} \text{ when } a > b)$ = $\frac{Pa}{l}$

M_{max} (at point of load) = $\frac{Pab}{l}$

M_x (when $x < a$) = $\frac{Pbx}{l}$

$\Delta_{y_{max}}$ (at $x = \sqrt{\frac{a(a+2b)}{3}}$ when $a > b$) = $\frac{Pab(a+2b)\sqrt{3a(a+2b)}}{27EI}$

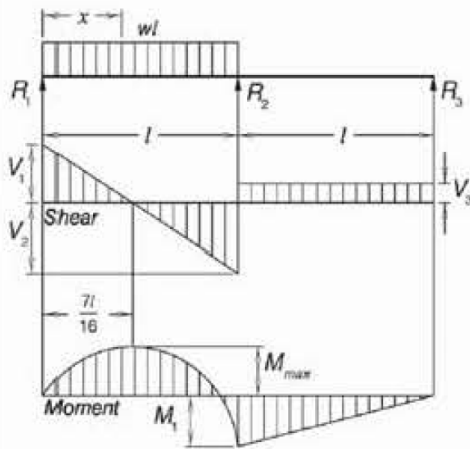
Δ_{θ} (at point of load) = $\frac{Pa^2 b^2}{3EI}$

Δ_x (when $x < a$) = $\frac{Pbx}{6EI}(l^2 - b^2 - x^2)$

Δ_x (when $x > a$) = $\frac{Pa(l-x)}{6EI}(2lx - x^2 - a^2)$

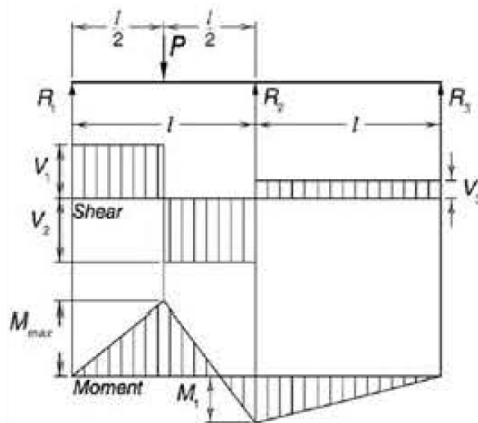
Δ_{x_1} = $\frac{Pabx_1}{6EI}(l+a)$

29. CONTINUOUS BEAM – TWO EQUAL SPANS – UNIFORM LOAD ON ONE SPAN



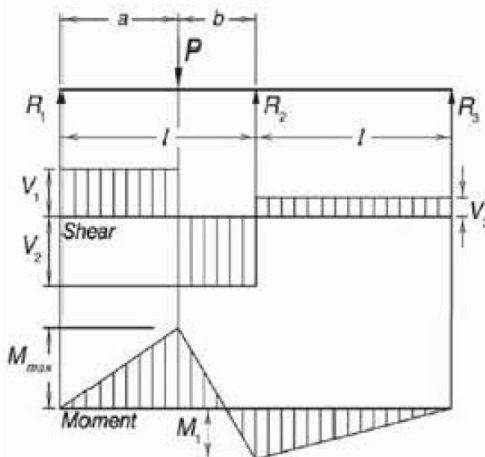
Total Equiv. Uniform Load	$= \frac{49}{64} wl$
$R_1 = V_1$	$= \frac{7}{16} wl$
$R_2 = V_2 + V_3$	$= \frac{5}{8} wl$
$R_3 = V_3$	$= -\frac{1}{16} wl$
V_2	$= \frac{9}{16} wl$
M_{max} (at $x = \frac{7}{16} l$)	$= \frac{49}{512} wl^2$
M_2 (at support R_2)	$= \frac{1}{16} wl^2$
M_x (when $x < l$)	$= \frac{wx}{16} (7l - 8x)$
Δ_{max} (at $0.472 l$ from R_1)	$= \frac{0.0092 wl^4}{EI}$

30. CONTINUOUS BEAM – TWO EQUAL SPANS – CONCENTRATED LOAD AT CENTER OF ONE SPAN



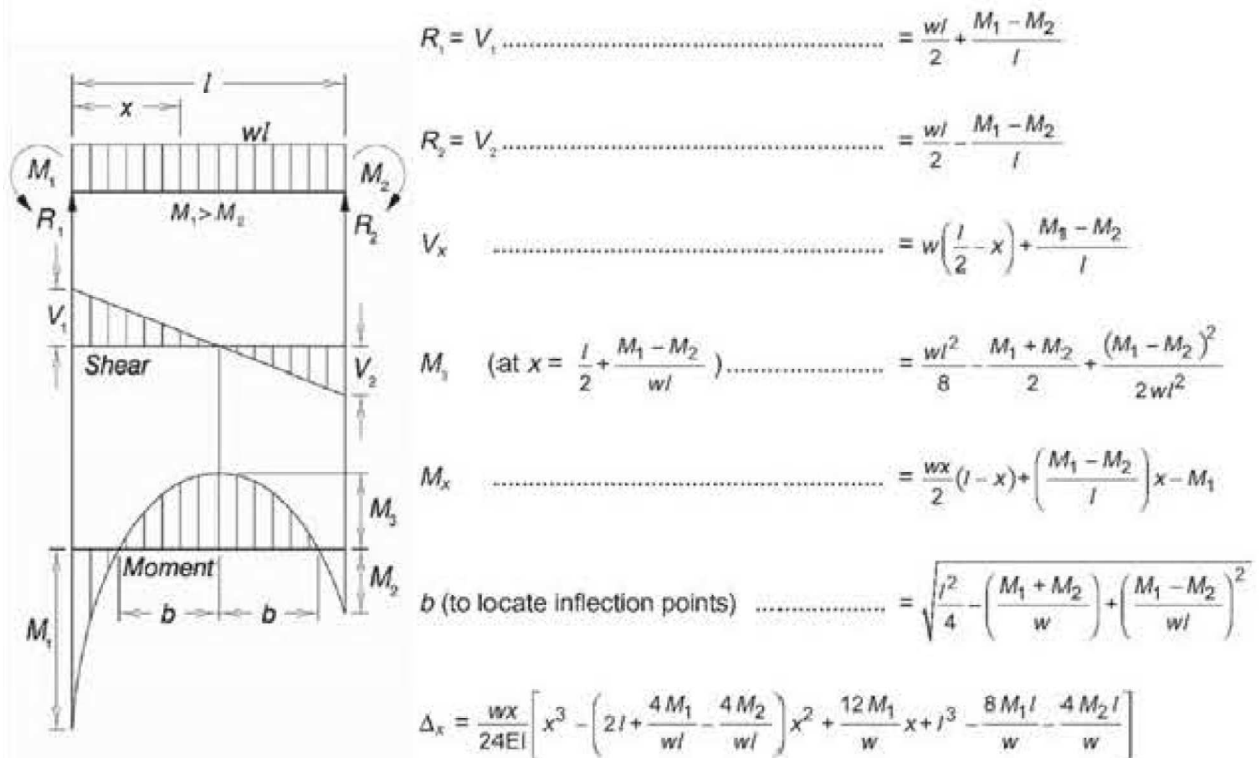
Total Equiv. Uniform Load	$= \frac{13}{8} P$
$R_1 = V_1$	$= \frac{13}{32} P$
$R_2 = V_2 + V_3$	$= \frac{11}{16} P$
$R_3 = V_3$	$= -\frac{3}{32} P$
V_2	$= \frac{19}{32} P$
M_{max} (at point of load)	$= \frac{13}{64} Pl$
M_2 (at support R_2)	$= \frac{3}{32} Pl$
Δ_{max} (at $0.480 l$ from R_1)	$= \frac{0.015 Pl^3}{EI}$

31. CONTINUOUS BEAM – TWO EQUAL SPANS – CONCENTRATED LOAD AT ANY POINT

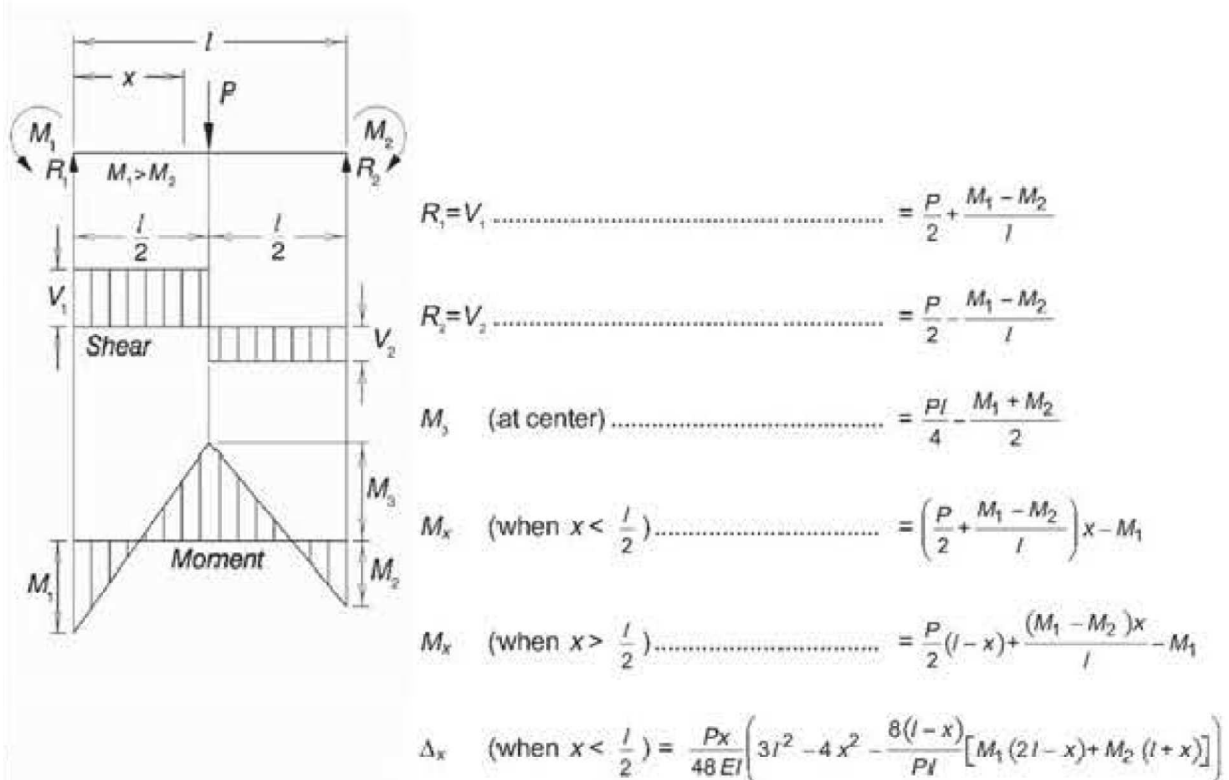


$R_1 = V_1$	$= \frac{Pb}{4l^3} (4l^2 - a(l+a))$
$R_2 = V_2 + V_3$	$= \frac{Pa}{2l^3} (2l^2 + b(l+a))$
$R_3 = V_3$	$= \frac{Pab}{4l^3} (l+a)$
V_2	$= \frac{Pa}{4l^3} (4l^2 + b(l+a))$
M_{max} (at point of load)	$= \frac{Pab}{4l^3} (4l^2 - a(l+a))$
M_1 (at support R_2)	$= \frac{Pab}{4l^2} (l+a)$

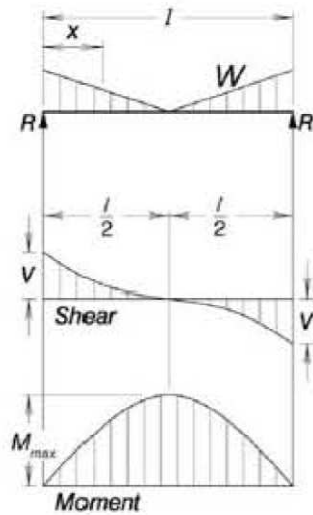
32. BEAM — UNIFORMLY DISTRIBUTED LOAD AND VARIABLE END MOMENTS



33. BEAM — CONCENTRATED LOAD AT CENTER AND VARIABLE END MOMENTS

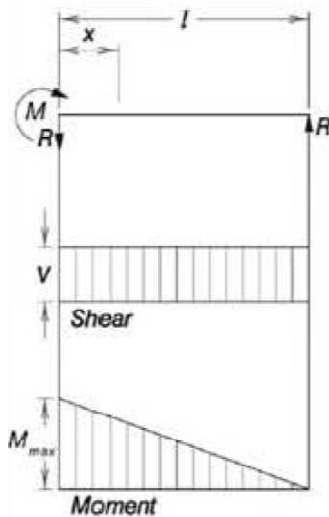


34. SIMPLE BEAM — LOAD INCREASING UNIFORMLY FROM CENTER



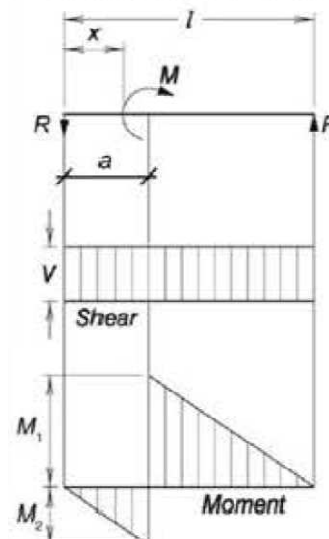
$$\begin{aligned} \text{Total Equiv. Uniform Load} &= \frac{2W}{3} \\ R=V &= \frac{W}{2} \\ V_x \text{ (when } x < \frac{l}{2}) &= \frac{W}{2} \left(\frac{l-2x}{l} \right)^2 \\ M_{max} \text{ (at center)} &= \frac{Wl}{12} \\ M_x \text{ (when } x < \frac{l}{2}) &= \frac{W}{2} \left(x - \frac{2x^2}{l} + \frac{4x^3}{3l^2} \right) \\ \Delta_{max} \text{ (at center)} &= \frac{3Wl^3}{320EI} \\ \Delta_x \text{ (when } x < \frac{l}{2}) &= \frac{W}{12EI} \left(x^3 - \frac{x^4}{l} + \frac{2x^5}{5l^2} - \frac{3l^2x}{8} \right) \end{aligned}$$

35. SIMPLE BEAM — CONCENTRATED MOMENT AT END



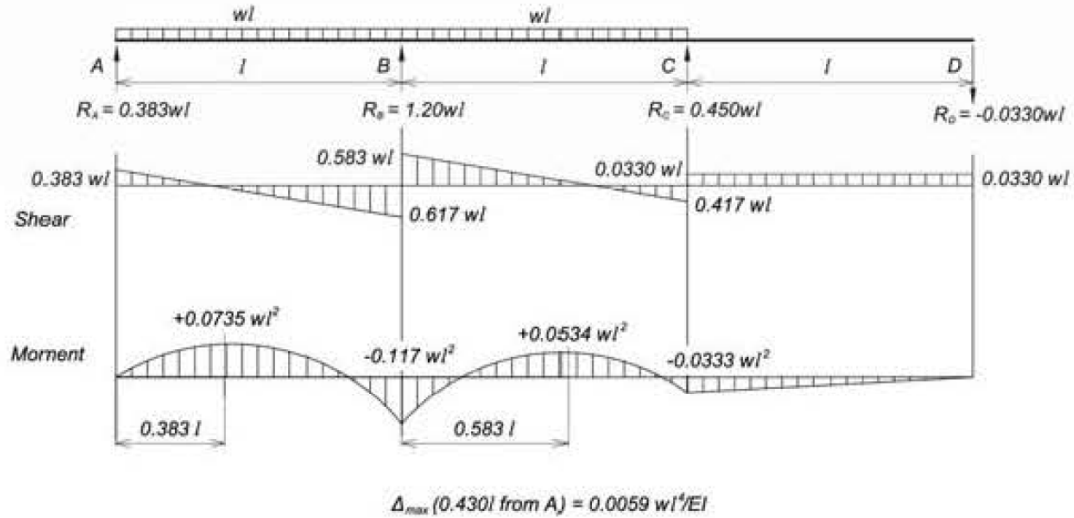
$$\begin{aligned} \text{Total Equiv. Uniform Load} &= \frac{8M}{l} \\ R=V &= \frac{M}{l} \\ M_{max} &= M \\ M_x &= M \left(1 - \frac{x}{l} \right) \\ \Delta_{max} \text{ (at } x = 0.423 l) &= 0.0642 \frac{Ml^2}{EI} \\ \Delta_x &= \frac{M}{6EI} \left(3x^2 - \frac{x^3}{l} - 2lx \right) \end{aligned}$$

36. SIMPLE BEAM — CONCENTRATED MOMENT AT ANY POINT

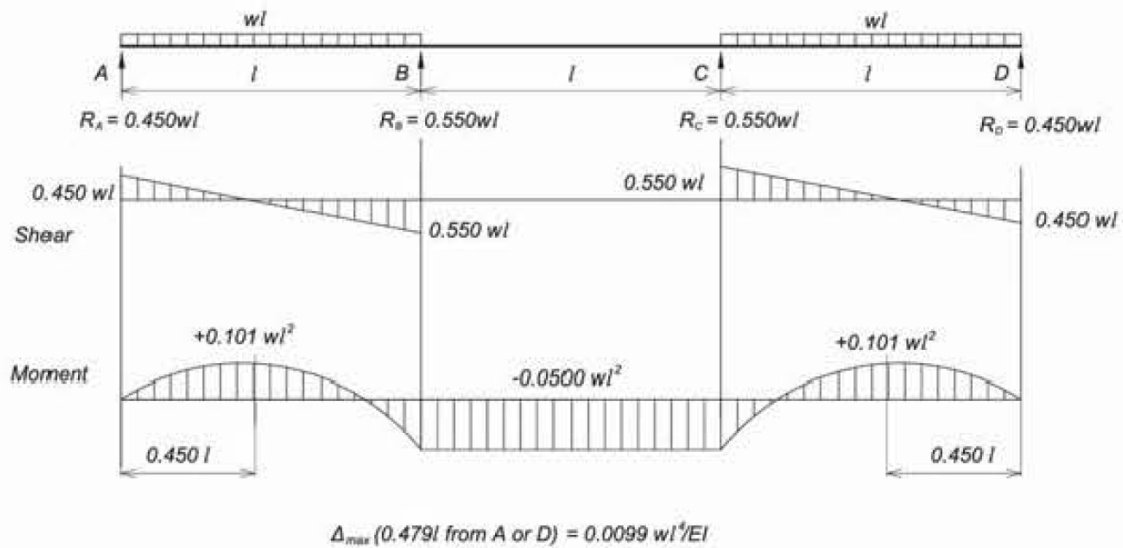


$$\begin{aligned} \text{Total Equiv. Uniform Load} &= \frac{8M}{l} \\ R=V &= \frac{M}{l} \\ M_x \text{ (when } x < a) &= Rx \\ M_x \text{ (when } x > a) &= R(l-x) \\ \Delta_x \text{ (when } x < a) &= \frac{M}{6EI} \left[\left(6a - \frac{3a^2}{l} - 2l \right) x - \frac{x^3}{l} \right] \\ \Delta_x \text{ (when } x > a) &= \frac{M}{6EI} \left[3(a^2 + x^2) - \frac{x^3}{l} - \left(2l + \frac{3a^2}{l} \right) x \right] \end{aligned}$$

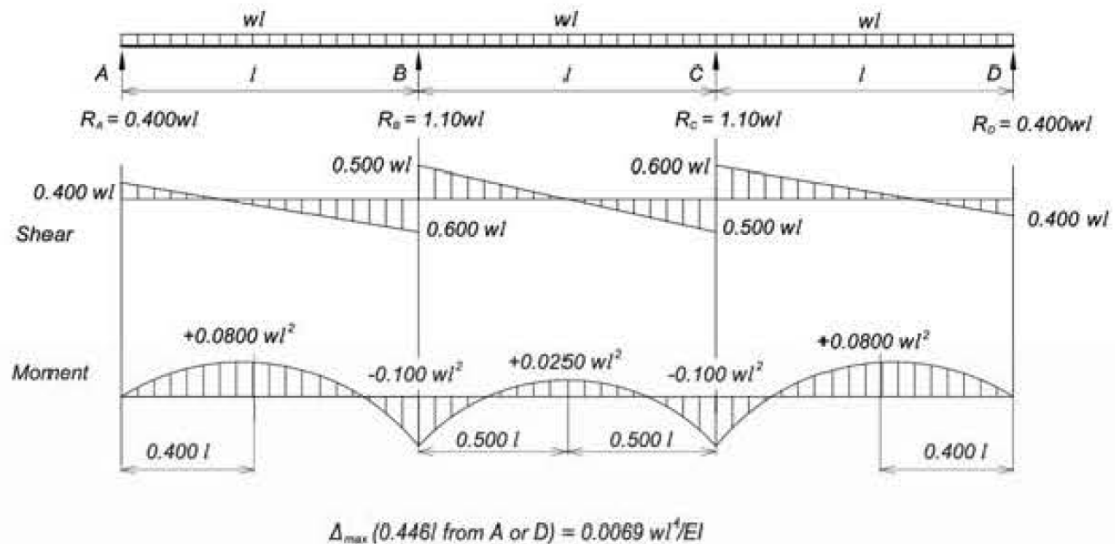
37. CONTINUOUS BEAM — THREE EQUAL SPANS — ONE END SPAN UNLOADED



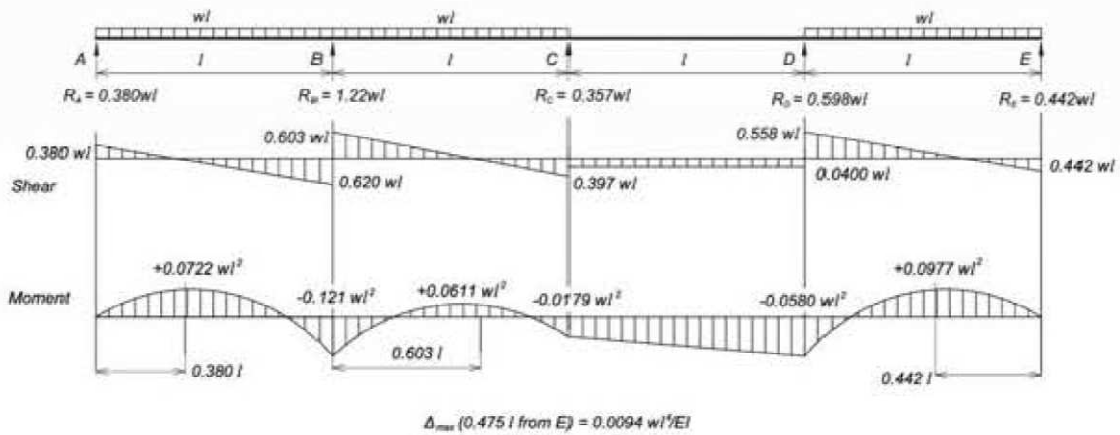
38. CONTINUOUS BEAM — THREE EQUAL SPANS — END SPANS LOADED



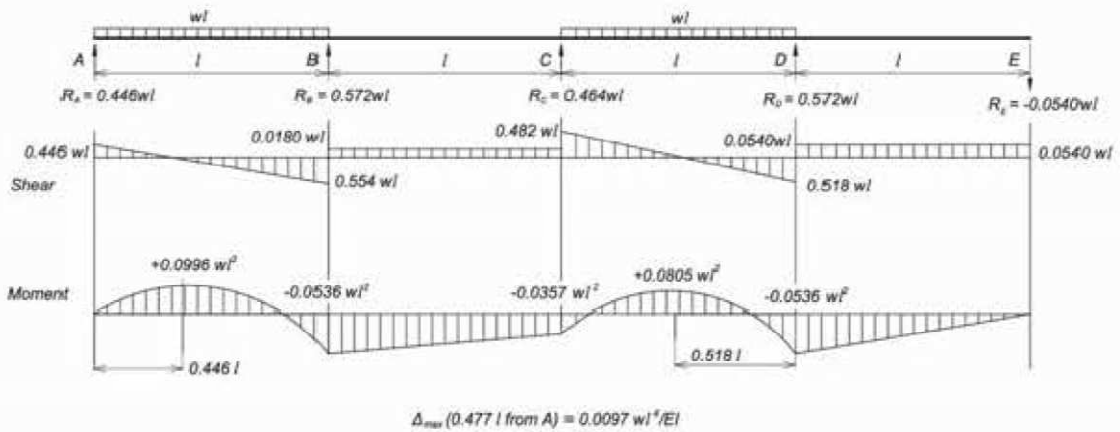
39. CONTINUOUS BEAM — THREE EQUAL SPANS — ALL SPANS LOADED



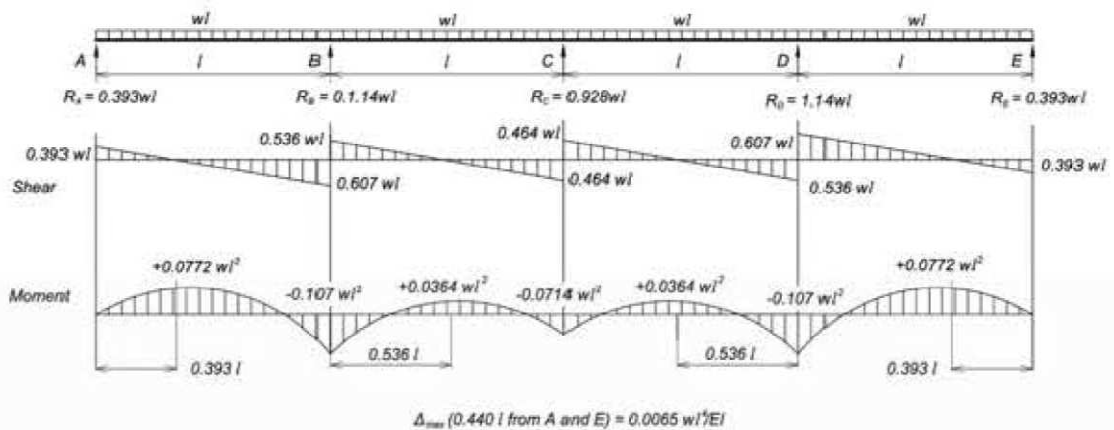
40. CONTINUOUS BEAM — FOUR EQUAL SPANS — THIRD SPAN UNLOADED



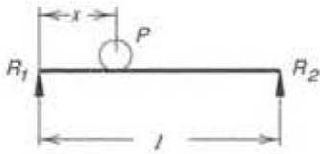
41. CONTINUOUS BEAM — FOUR EQUAL SPANS — LOAD FIRST AND THIRD SPANS



42. CONTINUOUS BEAM — FOUR EQUAL SPANS — ALL SPANS LOADED



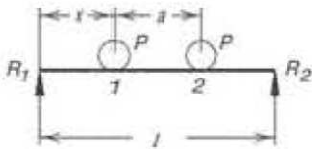
43. SIMPLE BEAM — ONE CONCENTRATED MOVING LOAD



$$R_{1\max} = V_{1\max} \text{ (at } x = 0) \dots\dots\dots = P$$

$$M_{\max} \text{ (at point of load, when } x = \frac{l}{2}) \dots\dots\dots = \frac{Pl}{4}$$

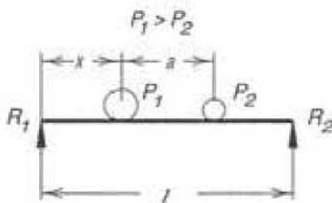
44. SIMPLE BEAM — TWO EQUAL CONCENTRATED MOVING LOADS



$$R_{1\max} = V_{1\max} \text{ (at } x = 0) \dots\dots\dots = P \left(2 - \frac{a}{l} \right)$$

$$M_{\max} \begin{cases} \left[\begin{array}{l} \text{when } a < (2 - \sqrt{2})l = 0.586l \\ \text{under load 1 at } x = \frac{1}{2} \left(l - \frac{a}{2} \right) \end{array} \right] \dots\dots\dots = \frac{P}{2l} \left(l - \frac{a}{2} \right)^2 \\ \left[\begin{array}{l} \text{when } a > (2 - \sqrt{2})l = 0.586l \\ \text{with one load at center of span (Case 43)} \end{array} \right] \dots\dots\dots = \frac{Pl}{4} \end{cases}$$

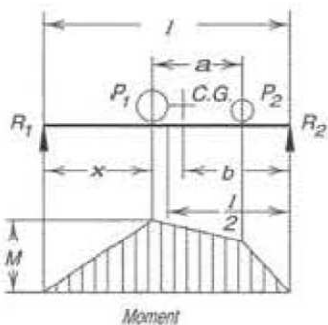
45. SIMPLE BEAM — TWO UNEQUAL CONCENTRATED MOVING LOADS



$$R_{1\max} = V_{1\max} \text{ (at } x = 0) \dots\dots\dots = P_1 + P_2 \frac{l-a}{l}$$

$$M_{\max} \begin{cases} \left[\text{under } P_1, \text{ at } x = \frac{1}{2} \left(l - \frac{P_2 a}{P_1 + P_2} \right) \right] \dots\dots\dots = (P_1 + P_2) \frac{x^2}{l} \\ \left[\begin{array}{l} M_{\max} \text{ may occur with larger} \\ \text{load at center of span and other} \\ \text{load off span (Case 43)} \end{array} \right] \dots\dots\dots = \frac{P_1 l}{4} \end{cases}$$

GENERAL RULES FOR SIMPLE BEAMS CARRYING MOVING CONCENTRATED LOADS



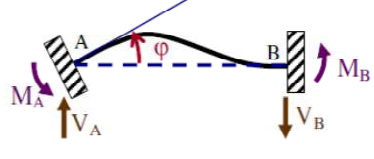
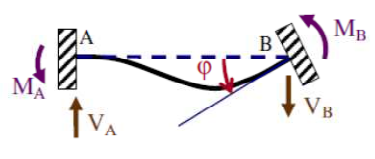
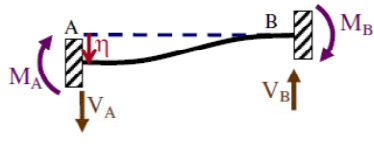
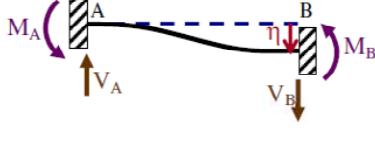


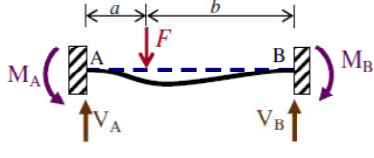
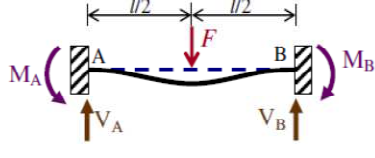
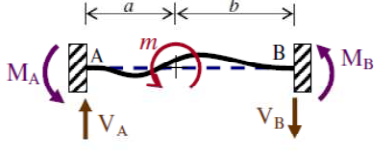
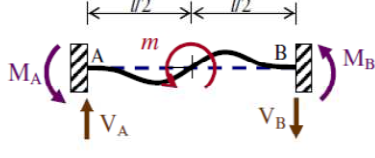
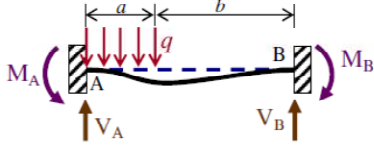

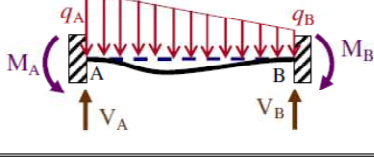
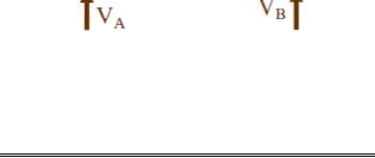


The maximum shear due to moving concentrated loads occurs at one support when one of the loads is at that support. With several moving loads, the location that will produce maximum shear must be determined by trial.

The maximum bending moment produced by moving concentrated loads occurs under one of the loads when that load is as far from one support as the center of gravity of all the moving loads on the beam is from the other support.

In the accompanying diagram, the maximum bending moment occurs under load P_1 when $x = b$. It should also be noted that this condition occurs when the center-line of the span is midway between the center of gravity of loads and the nearest concentrated load.

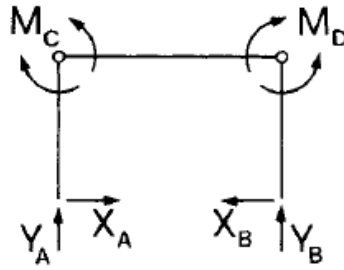
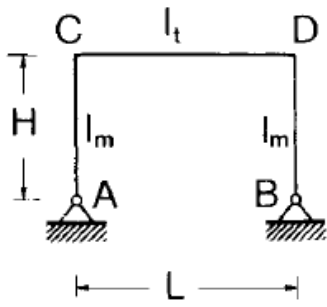
Reactions in end fixed beams

Note: l = beam length, h = transversal section height, A = area, I = inertia moment, E = Young modulus, α = thermal dilatation coefficient. Positive values for the expression of the reactions corresponds to the sign assumed in the figure for the reactions.

	$M_A = \frac{4EI}{l} \phi$ $M_B = \frac{2EI}{l} \phi$ $V_A = \frac{6EI}{l^2} \phi$ $V_B = \frac{6EI}{l^2} \phi$		$M_A = \frac{2EI}{l} \phi$ $M_B = \frac{4EI}{l} \phi$ $V_A = \frac{6EI}{l^2} \phi$ $V_B = \frac{6EI}{l^2} \phi$
	$M_A = \frac{6EI}{l^2} \eta$ $M_B = \frac{6EI}{l^2} \eta$ $V_A = \frac{12EI}{l^3} \eta$ $V_B = \frac{12EI}{l^3} \eta$		$M_A = \frac{6EI}{l^2} \eta$ $M_B = \frac{6EI}{l^2} \eta$ $V_A = \frac{12EI}{l^3} \eta$ $V_B = \frac{12EI}{l^3} \eta$
	$H_A = \frac{EA}{l} \eta$ $H_B = \frac{EA}{l} \eta$		$H_A = \frac{EA}{l} \eta$ $H_B = \frac{EA}{l} \eta$
	$M_A = \frac{ab^2}{l^2} F$ $M_B = \frac{ba^2}{l^2} F$ $V_A = \frac{b^2}{l^3} (l+2a) F$ $V_B = \frac{b^2}{l^3} (l+2b) F$		$M_A = \frac{Fl}{8}$ $M_B = \frac{Fl}{8}$ $V_A = \frac{F}{2}$ $V_B = \frac{F}{2}$
	$M_A = \frac{b}{l^2} (2a-b) m$ $M_B = \frac{a}{l^2} (2b-a) m$ $V_A = \frac{6ab}{l^3} m$ $V_B = \frac{6ab}{l^3} m$		$M_A = \frac{m}{4}$ $M_B = \frac{m}{4}$ $V_A = \frac{3m}{2l}$ $V_B = \frac{3m}{2l}$
	$M_A = \frac{qa^2}{12l^2} (l^2 + 2lb + 3b^2)$ $M_B = \frac{qa^2}{12l^2} (l + 3b)$ $V_A = \frac{qa}{2l^3} [2l^3 - a^2(l+b)]$ $V_B = \frac{qa^3}{2l^3} (l+b)$		$M_A = \frac{ql^2}{12}$ $M_B = \frac{ql^2}{12}$ $V_A = \frac{ql}{2}$ $V_B = \frac{ql}{2}$
	$M_A = \frac{l^2}{60} (3q_A + 2q_B)$ $M_B = \frac{l^2}{60} (2q_A + 3q_B)$ $V_A = \frac{l}{20} (7q_A + 3q_B)$ $V_B = \frac{l}{20} (3q_A + 7q_B)$		$M_A = \frac{ql^2}{12}$ $M_B = \frac{ql^2}{12}$ $V_A = \frac{ql}{2}$ $V_B = \frac{ql}{2}$
	$M_A = 2EI\alpha \frac{\Delta T}{h}$ $M_B = 2EI\alpha \frac{\Delta T}{h}$		$H_A = EA\alpha\Delta T$ $H_B = EA\alpha\Delta T$

BEAM AND FRAME SCHEMES

a) SUPPORTED FRAME

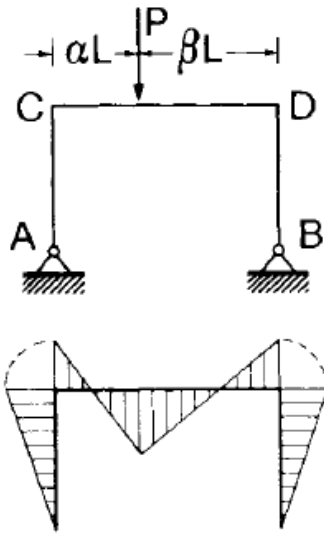


$$K_1 = \frac{H}{L} \frac{I_t}{I_m}$$

$$K_2 = 2 \left(1 + \frac{2}{3} K_1 \right)$$

Legenda

1.

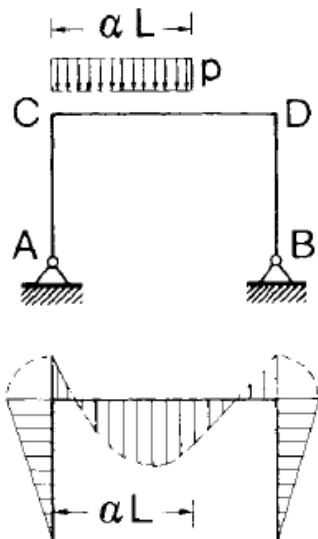


$$X_A = X_B = P \frac{L}{H} \frac{\alpha \beta}{K_2}$$

$$Y_A = P \beta \quad Y_B = P \alpha$$

$$M_C = M_D = PL \frac{\alpha \beta}{K_2}$$

2.

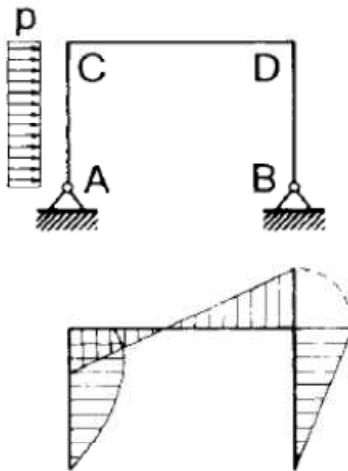


$$X_A = X_B = p \frac{\alpha^2 L^2}{6H} \frac{3-2\alpha}{K_2}$$

$$Y_A = p \alpha L \frac{2-\alpha}{2} \quad Y_B = p \alpha^2 \frac{L}{2}$$

$$M_C = M_D = p \frac{\alpha^2 L^2}{6} \frac{3-2\alpha}{K_2}$$

3.

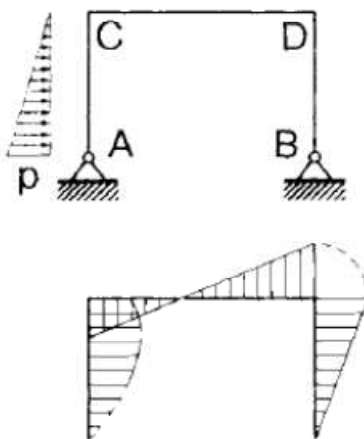


$$X_A = -p \frac{H}{4} \left(3 - \frac{K_1}{3K_2} \right) \quad X_B = +p \frac{H}{4} \left(1 + \frac{K_1}{3K_2} \right)$$

$$Y_A = -Y_B = -p \frac{H^2}{2L}$$

$$M_C = -p \frac{H^2}{4} \left(1 - \frac{K_1}{3K_2} \right) \quad M_D = +p \frac{H^2}{4} \left(1 + \frac{K_1}{3K_2} \right)$$

4.

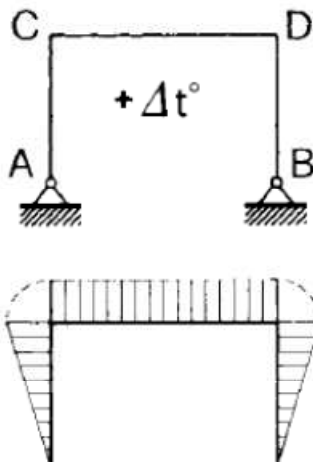


$$X_A = -p \frac{H}{180} \left(75 - 7 \frac{K_1}{K_2} \right) \quad X_B = +p \frac{H}{180} \left(15 + 7 \frac{K_1}{K_2} \right)$$

$$Y_A = -Y_B = -\frac{pH^2}{6L}$$

$$M_C = -p \frac{H^2}{180} \left(15 - 7 \frac{K_1}{K_2} \right) \quad M_D = +p \frac{H^2}{180} \left(15 + 7 \frac{K_1}{K_2} \right)$$

5.

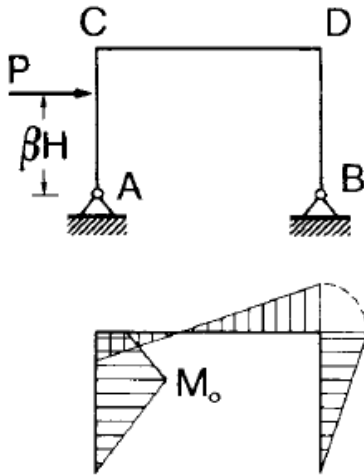


$$X_A = X_B = \frac{2 E \alpha \Delta t I_1}{K_2 H^2}$$

$$Y_A = Y_B = 0$$

$$M_C = M_D = \frac{2 E \alpha \Delta t I_1}{K_2 H}$$

6.



$$\chi = \frac{1 + K_1 (1 - 1/3 \beta^2)}{K^2}$$

$$X_A = -P (1 - \beta \chi)$$

$$X_B = P \beta \chi$$

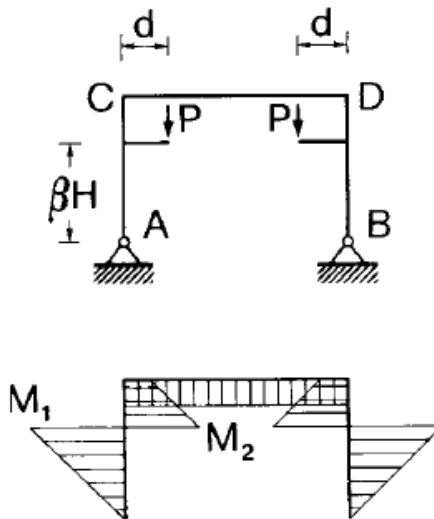
$$Y_B = -Y_A = P \frac{H}{L} \beta$$

$$M_C = -P \beta H (1 - \chi)$$

$$M_D = P \beta H \chi$$

$$M_0 = -X_A \beta H$$

7.



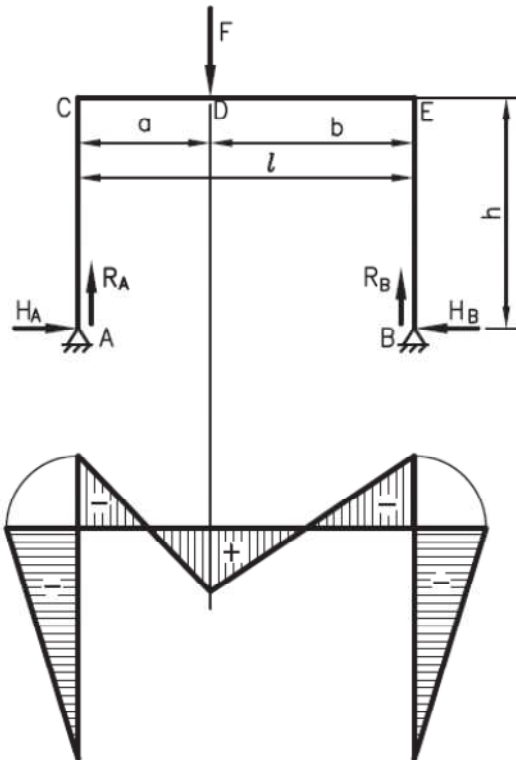
$$X_A = X_B = 2 \frac{Pd}{H} \frac{1 + K_1 (1 - \beta^2)}{K_2}$$

$$Y_A = Y_B = P$$

$$M_C = M_D = -Pd + X_A H$$

$$M_1 = X_A \beta H$$

$$M_2 = Pd - X_A \beta H$$



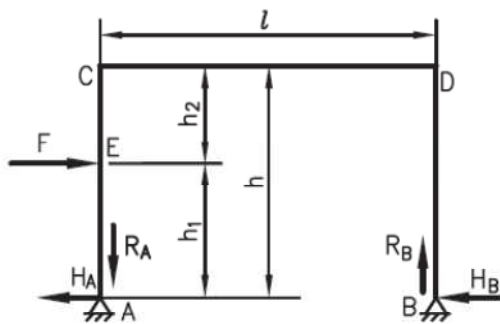
$$R_A = \frac{F \cdot b}{l}; \quad R_B = \frac{F \cdot a}{l}$$

$$k = \frac{h}{l}$$

$$H_A = H_B = \frac{3 \cdot F \cdot a \cdot b}{2 \cdot h \cdot l \cdot (2k + 3)}$$

$$M_C = M_E = \frac{3 \cdot F \cdot a \cdot b}{2 \cdot l \cdot (2k + 3)}$$

$$M_D = \frac{F \cdot a \cdot b}{2l} \cdot \frac{4k + 3}{2k + 3}$$



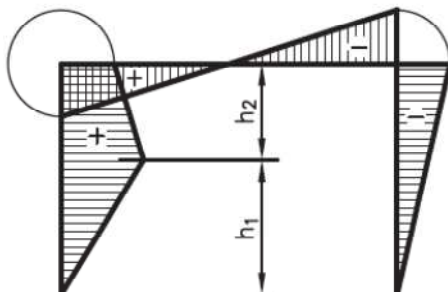
$$R_A = R_B = \frac{F \cdot h_1}{l}$$

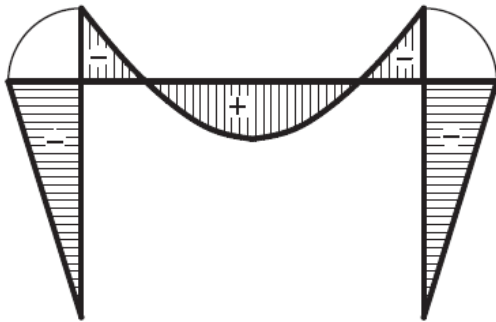
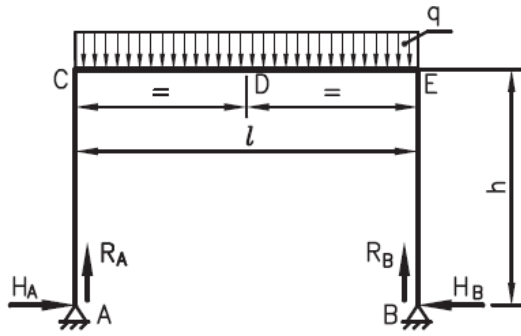
$$H_A = \frac{F \cdot h}{2 \cdot l} \cdot \left[\frac{4h^3 + h_1^3 - 3h_1 \cdot h^2}{h \cdot (2h + 3l)} + \frac{6h \cdot l - 3h_1 \cdot h \cdot l}{h \cdot (2h + 3l)} \right]$$

$$H_B = F - H_A$$

$$M_E = H_A \cdot h; \quad M_C = H_A \cdot h - F \cdot h_2$$

$$M_D = -H_B \cdot h = -(F - H_A) \cdot h$$



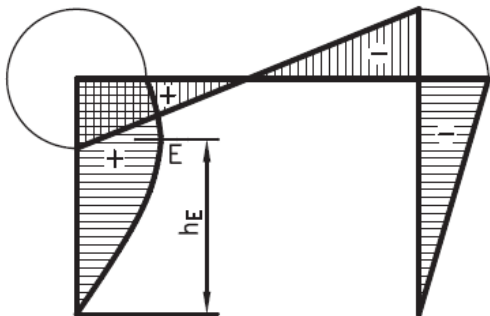
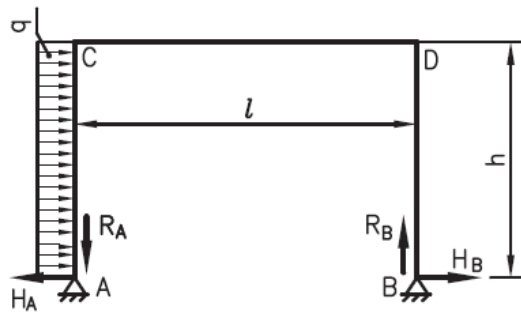


$$R_A = R_B = \frac{q \cdot l}{2}$$

$$H_A = H_B = \frac{q \cdot l^2}{4h \cdot \left(2\frac{h}{l} + 3\right)}$$

$$M_D = \frac{q \cdot l^2}{8} \cdot \frac{2h+l}{2h+3l}$$

$$M_C = M_E = \frac{q \cdot l^2}{4 \cdot \left(2\frac{h}{l} + 3\right)}$$



$$R_A = R_B = \frac{q \cdot h^2}{2l}$$

$$H_A = \frac{q \cdot h}{8} \cdot \frac{11h+18l}{2h+3l}$$

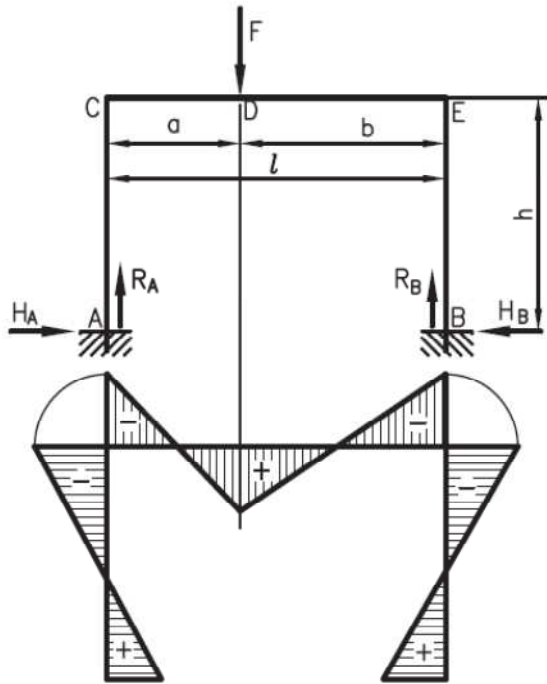
$$H_B = q \cdot h - H_A$$

$$M_E = \frac{q \cdot h^2}{128} \cdot \left(\frac{11k+18}{2k+3}\right)^2; \quad k = \frac{h}{l}$$

$$h_E = \frac{h}{8} \cdot \left(\frac{11k+18}{2k+3}\right)$$

$$M_C = \frac{3}{8} \cdot q \cdot l^2 \cdot \frac{h+2l}{2h+3l}$$

$$M_D = -\frac{q \cdot h^2}{8} \cdot \frac{5k+6}{2k+3}$$



$$R_A = \frac{F \cdot b}{l} \cdot \frac{6 \cdot h \cdot l + l^2 + a \cdot l - 2a^2}{6 \cdot h \cdot l + l^2}$$

$$R_B = \frac{F \cdot a}{l} \cdot \frac{6 \cdot h \cdot l + 3 \cdot a \cdot l - 2a^2}{6 \cdot h \cdot l + l^2}$$

$$H_A = H_B = \frac{3 \cdot F \cdot a \cdot b}{2 \cdot h \cdot l \cdot \left(\frac{h}{l} + 2\right)}$$

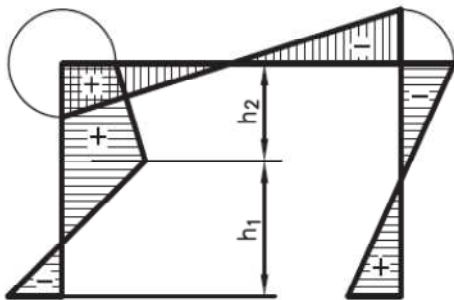
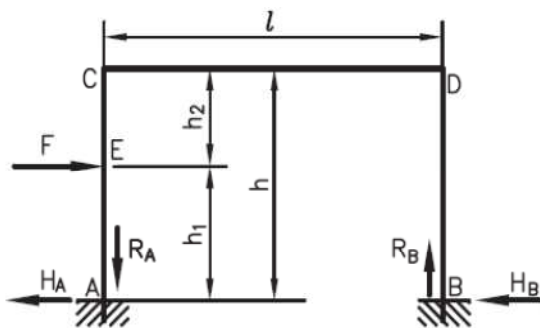
$$M_A = \frac{F \cdot a \cdot b}{2l} \cdot \frac{5 \cdot h \cdot l - l^2 + 2a \cdot (h + 2l)}{(h + 2l) \cdot (6h + l)}$$

$$M_B = \frac{F \cdot a \cdot b}{2l} \cdot \frac{3l + 7 \cdot h \cdot l - 2a \cdot (h + 2l)}{(h + 2l) \cdot (6h + l)}$$

$$M_C = M_A - H_A \cdot h$$

$$M_E = M_B - H_B \cdot h$$

$$M_D = M_A - H_A \cdot h + R_A \cdot a$$



$$R_A = R_B = \frac{3 \cdot F \cdot h_1^2}{6 \cdot h \cdot l + l^2}; \quad H_A = F - H_B$$

$$H_B = \frac{F \cdot h_1}{2 \cdot h^2 \cdot \left(\frac{h}{l} + 2\right)} \cdot \left[3 \cdot \left(\frac{h}{l} + 1\right) - \frac{h_1}{h} \cdot \left(2 \cdot \frac{h}{l} + 1\right) \right]$$

$$M_A = -\frac{F \cdot h_1^2}{2h} \cdot \left[\frac{2h}{h_1} - \frac{3h \cdot l + 2h^2 - h \cdot (h + l)}{h^2 + 2 \cdot h \cdot l} - \frac{3h^2}{6 \cdot h^2 + h \cdot l} \right]$$

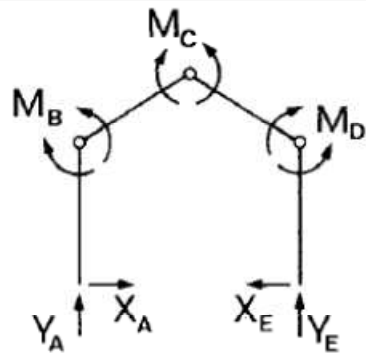
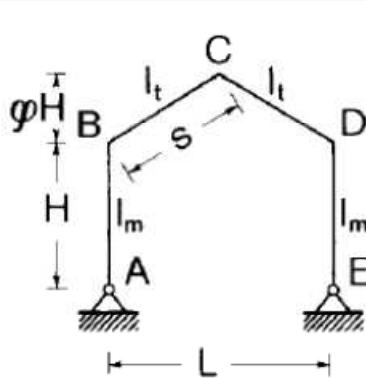
$$M_B = \frac{F \cdot h_1^2}{2 \cdot h} \cdot \left[\frac{3 \cdot h \cdot l + 2h^2 - h \cdot (h + l)}{h^2 + 2 \cdot h \cdot l} - \frac{3h^2}{6 \cdot h^2 + h \cdot l} \right]$$

$$M_E = M_A + H_A \cdot h_1$$

$$M_D = M - H_B \cdot h$$

$$M_C = M_A + H_A \cdot h_1 - F \cdot h_2$$

b) SUPPORTED FRAME WITH SLOPED ROOF

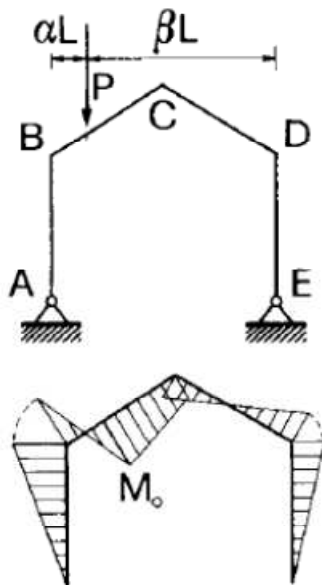


$$K_1 = \frac{H}{S} \frac{I_t}{I_m}$$

$$K_2 = (3 + K_1) + \phi (3 + \phi)$$

Legenda

1.



$$X_A = X_E = \frac{P \alpha L}{4H} \frac{1}{K_2} [6\beta + \phi (3 - 4 \alpha^2)]$$

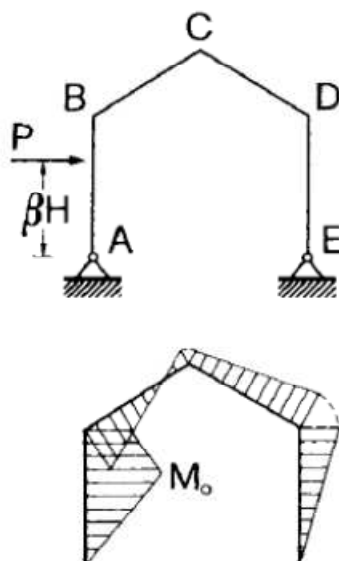
$$Y_A = P \beta \quad Y_E = P \alpha$$

$$M_B = M_D = X_A H$$

$$M_C = -\frac{P \alpha L}{2} + X_A H (1 + \phi)$$

$$M_0 = Y_A \alpha L - X_A H (1 + 2 \phi \alpha)$$

2.



$$X_E = \frac{P \beta}{4} \frac{1}{K_2} [K_1 (3 - \beta^2) + 3 (2 + \phi)]$$

$$X_A = X_E - P$$

$$Y_E = -Y_A = P \beta H/L$$

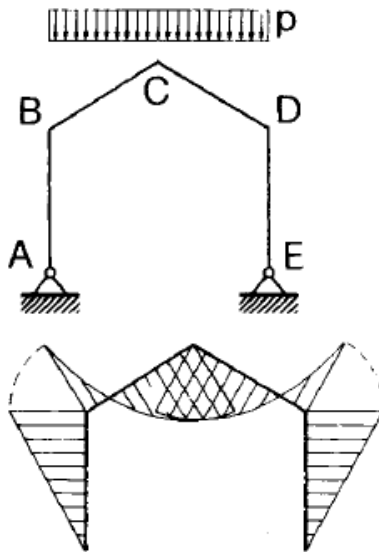
$$M_B = X_E H - P \beta H$$

$$M_C = X_E H (1 + \phi) - P \beta \frac{H}{2}$$

$$M_D = X_E H$$

$$M_0 = -X_A \beta H$$

3.



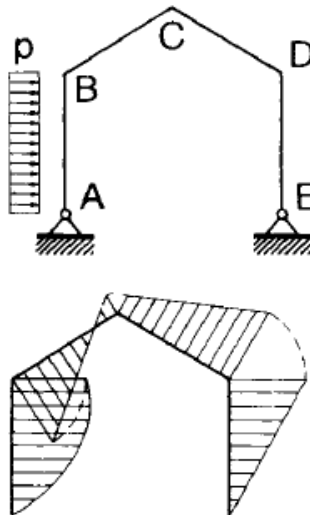
$$X_A = X_E = \frac{pL^2}{32H} \frac{1}{K_2} (8 + 5 \varphi)$$

$$Y_A = Y_E = P \frac{L}{2}$$

$$M_B = M_D = X_A H$$

$$M_C = X_A H (1 + \varphi) - p \frac{L^2}{8}$$

4.



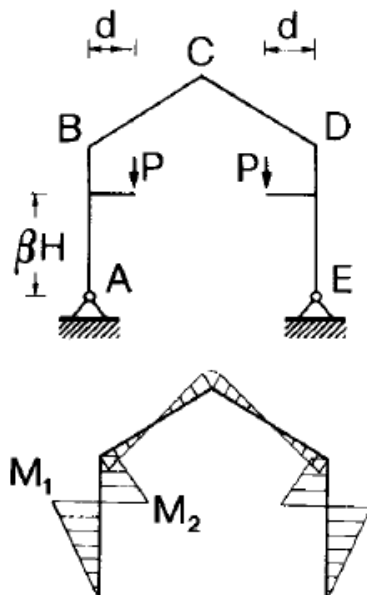
$$X_E = p \frac{H}{16} \frac{1}{K_2} [5 K_1 + 6 (1 + \varphi)] \quad X_A = X_E - p H$$

$$Y_E = -Y_A = p \frac{H^2}{2L}$$

$$M_B = X_E H - p \frac{H^2}{2} \quad M_C = X_E H (1 + \varphi) - p \frac{H^2}{4}$$

$$M_D = X_E H$$

5.



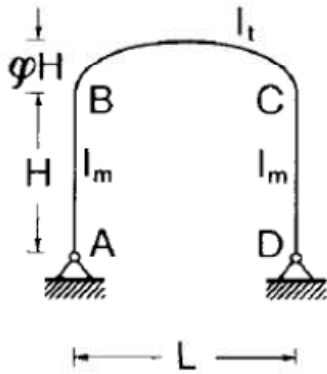
$$X_A = X_E = \frac{3}{2} \frac{Pd}{H} \frac{1}{K_2} [K_1 (1 - \beta^2) + (2 + \varphi)]$$

$$Y_A = Y_E = P$$

$$M_B = M_D = X_A H - Pd \quad M_C = X_A H (1 + \varphi) - Pd$$

$$M_1 = X_A \beta H \quad M_2 = Pd - X_A \beta H$$

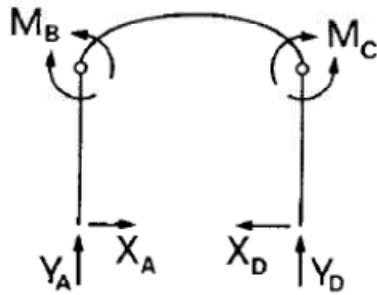
c) SUPPORTED FRAME WITH CURVED ROOF



$$K_1 = \frac{H}{L} \frac{I_t}{I_m}$$

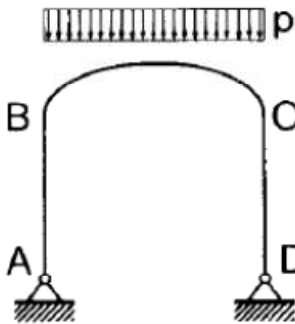
$$K_2 = 5(2K_1 + 3) + 4\phi(5 + 2\phi)$$

$$\phi \ll 1$$



Legenda

1.

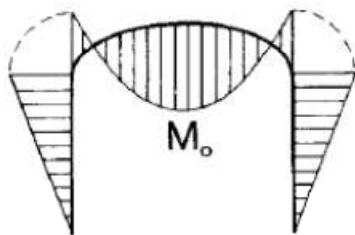


$$X_A = X_D = p \frac{L^2}{4H} \frac{1}{K_2} (5 + 4\phi)$$

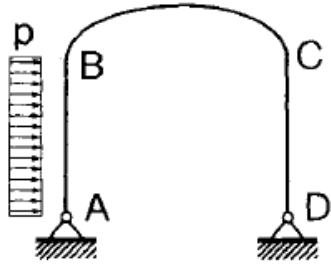
$$Y_A = Y_D = p \frac{L}{2}$$

$$M_B = M_C = X_A H$$

$$M_0 = p \frac{L^2}{8} - X_A H (1 + \phi)$$

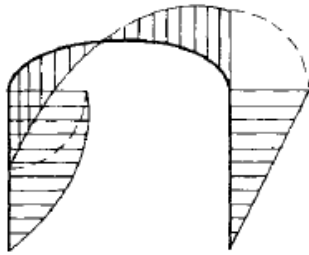


2.



$$X_D = \frac{5}{8} p H \frac{1}{K_2} [(5 K_1 + 6) + 4 \phi]$$

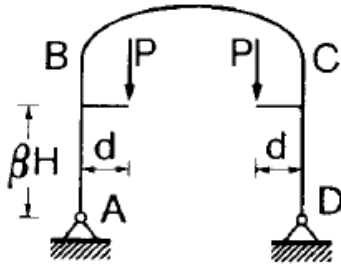
$$X_A = X_D - p H \qquad Y_A = Y_D = p \frac{H^2}{2L}$$



$$M_B = X_D H - p \frac{H^2}{2}$$

$$M_C = X_D H$$

3.



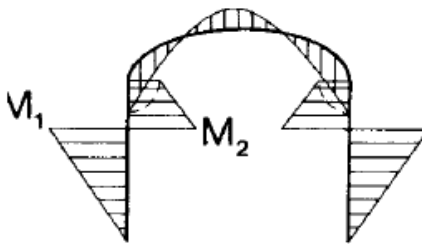
$$X_A = X_D = 5 \frac{P d}{H} \frac{1}{K_2} [3 K_1 (1 - \beta^2) + (3 + 2 \phi)]$$

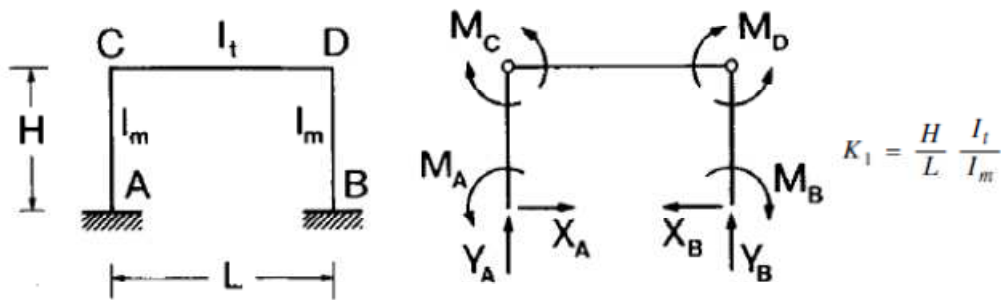
$$Y_A = Y_D = P$$

$$M_B = M_C = X_A H - P d$$

$$M_1 = X_A \beta H$$

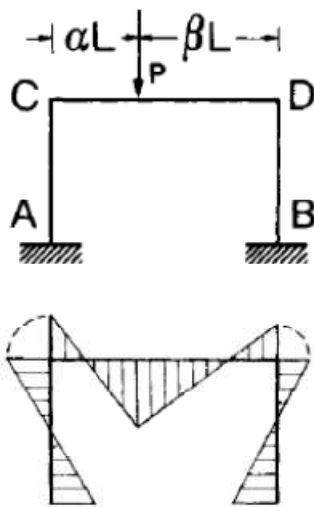
$$M_2 = P d - X_A \beta H$$





Legenda

1.



$$\chi_1 = \frac{1}{2+K_1} \quad \chi_2 = \frac{\beta-\alpha}{1+6K_1}$$

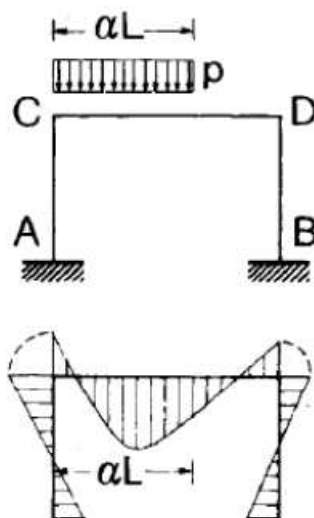
$$X_A = X_B = \frac{3}{2} P \frac{L}{H} \alpha \beta \chi_1$$

$$Y_A = P \beta (1 + \alpha \chi_2) \quad Y_B = P \alpha (1 - \beta \chi_2)$$

$$M_A = -P \frac{L}{2} \alpha \beta (\chi_1 - \chi_2) \quad M_B = -P \frac{L}{2} \alpha \beta (\chi_1 + \chi_2)$$

$$M_C = +P \frac{L}{2} \alpha \beta (2\chi_1 + \chi_2) \quad M_D = +P \frac{L}{2} \alpha \beta (2\chi_1 - \chi_2)$$

2.



$$\chi_1 = \frac{3-2\alpha}{2+K_1} \quad \chi_2 = \frac{(1-\alpha)^2}{1+6K_1}$$

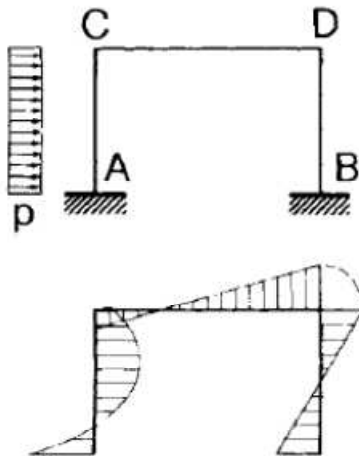
$$X_A = X_B = p \frac{\alpha^2 L^2}{4H} \chi_1$$

$$Y_A = p \alpha L \left[1 - \frac{\alpha}{2} (1 - \chi_2) \right] \quad Y_B = p \alpha^2 \frac{L}{2} (1 - \chi_2)$$

$$M_A = -p \frac{\alpha^2 L^2}{12} (\chi_1 - 3 \chi_2) \quad M_B = -p \frac{\alpha^2 L^2}{12} (\chi_1 + 3 \chi_2)$$

$$M_C = +p \frac{\alpha^2 L^2}{12} (2\chi_1 + 3 \chi_2) \quad M_D = +p \frac{\alpha^2 L^2}{12} (2\chi_1 - 3 \chi_2)$$

3.



$$\chi_1 = \frac{9+3K_1}{2+K_1} \quad \chi_2 = \frac{12K_1}{1+6K_1} \quad \chi_3 = \frac{K_1}{2+K_1}$$

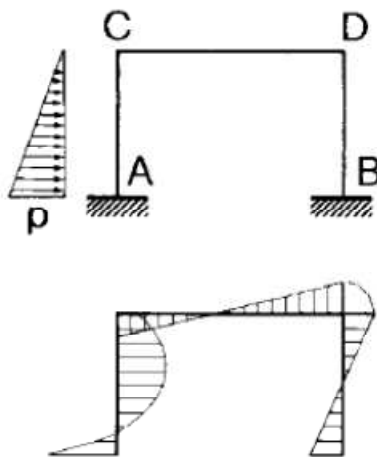
$$X_A = -p \frac{H}{8} \frac{13+6K_1}{2+K_1} \quad X_B = p \frac{H}{8} \frac{3+2K_1}{2+K_1}$$

$$Y_A = -Y_B = -p \frac{H^2}{L} \frac{K_1}{1+6K_1}$$

$$M_A = p \frac{H^2}{2} - p \frac{H^2}{24} (\chi_1 + \chi_2) \quad M_B = -p \frac{H^2}{24} (\chi_1 - \chi_2)$$

$$M_C = +p \frac{H^2}{24} (\chi_3 - \chi_2) \quad M_D = +p \frac{H^2}{24} (\chi_3 + \chi_2)$$

4.



$$\chi_1 = \frac{12+7K_1}{2+K_1} \quad \chi_2 = \frac{15K_1}{1+6K_1} \quad \chi_3 = \frac{2K_1}{2+K_1}$$

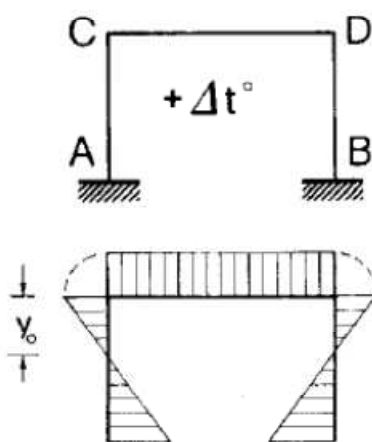
$$X_A = -p \frac{H}{40} \frac{36+17K_1}{2+K_1} \quad X_B = p \frac{H}{40} \frac{4+3K_1}{2+K_1}$$

$$Y_A = -Y_B = -p \frac{H^2}{4L} \frac{K_1}{1+6K_1}$$

$$M_A = p \frac{H^2}{6} - p \frac{H^2}{120} (\chi_1 + \chi_2) \quad M_B = -p \frac{H^2}{120} (\chi_1 - \chi_2)$$

$$M_C = +p \frac{H^2}{120} (\chi_3 - \chi_2) \quad M_D = +p \frac{H^2}{120} (\chi_3 + \chi_2)$$

5.



$$X_A = X_B = \frac{3E \alpha \Delta t I_t}{H^2} \frac{1+2K_1}{K_1 (2+K_1)}$$

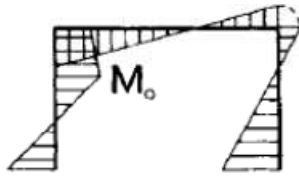
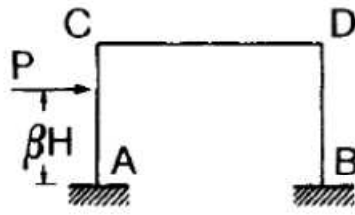
$$Y_A = Y_B = 0$$

$$M_A = M_B = -\frac{3E \alpha \Delta t I_t}{H} \frac{1+K_1}{K_1 (2+K_1)}$$

$$M_C = M_D = +\frac{3E \alpha \Delta t I_t}{H} \frac{1}{2+K_1}$$

$$y_0 = H \frac{K_1}{1+2K_1}$$

6.



$$\chi_1 = \frac{1 + (2 - \beta)(1 + K_1)}{2 + K_1}$$

$$\chi_2 = \frac{3K_1}{1 + 6K_1}$$

$$\chi_3 = \frac{(1 - \beta)K_1}{2 + K_1}$$

$$\chi_4 = \frac{3(1 + K_1) - \beta(1 + 2K_1)}{2 + K_1}$$

$$X_A = -P + \frac{P}{2} \beta^2 \chi_4$$

$$X_B = \frac{P}{2} \beta^2 \chi_4$$

$$Y_A = -Y_B = -P \frac{H}{L} \beta^2 \chi_2$$

$$M_A = PH \beta - \frac{PH}{2} \beta^2 (\chi_1 + \chi_2)$$

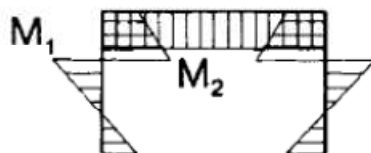
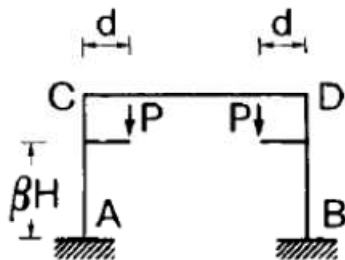
$$M_B = -\frac{PH}{2} \beta^2 (\chi_1 - \chi_2)$$

$$M_C = \frac{PH}{2} \beta^2 (\chi_3 - \chi_2)$$

$$M_D = P \frac{H}{2} \beta^2 (\chi_3 + \chi_2)$$

$$M_0 = -M_A - X_A \beta H$$

7.



$$X_A = X_B = 3 \frac{Pd}{H} \frac{2\beta K_1 (1 - \beta) + \beta (2 - \beta)}{2 + K_1}$$

$$Y_A = Y_B = P$$

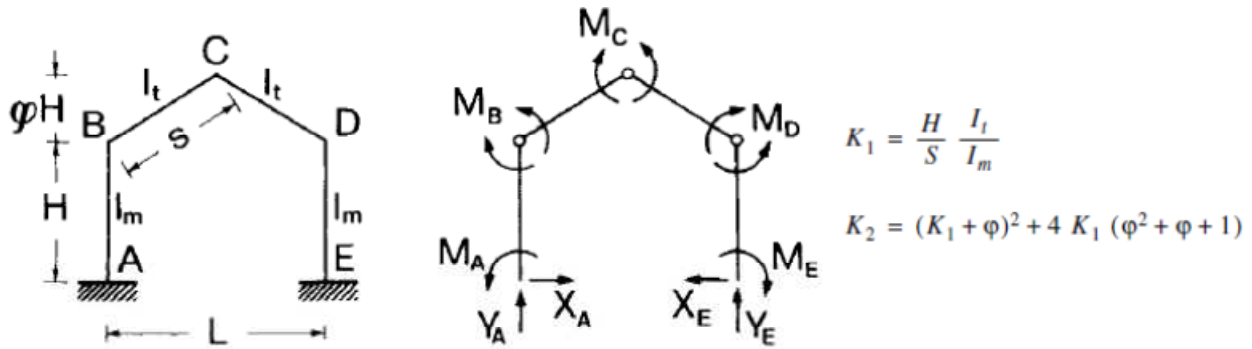
$$M_A = M_B = -Pd \frac{K_1 (4\beta - 3\beta^2 - 1) + (6\beta - 3\beta^2 - 2)}{2 + K_1}$$

$$M_C = M_D = M_A + X_A H - Pd$$

$$M_1 = M_A + X_A \beta H$$

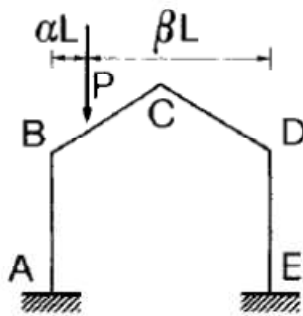
$$M_2 = -M_A - X_A \beta H + Pd$$

e) FIXED FRAME WITH SLOPED ROOF



Legenda

1.



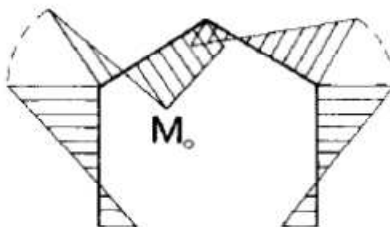
$$\chi_1 = \frac{1}{K_2} [2\beta K_1 + 3\phi (2\alpha + K_1) - \phi^2 (1 + 4\alpha) + 4\alpha^2 \phi (2 + K_1) - 4\alpha^2 \phi^2]$$

$$\chi_2 = \beta (\beta - \alpha) / (1 + 3K_1)$$

$$X_A = X_E = P \alpha \frac{L}{H} \frac{1}{K_2} [3K_1 (1 + \phi) + 4\alpha^2 \phi (1 + K_1) - 3\alpha (K_1 - \phi)]$$

$$Y_A = P \beta + (M_A - M_E) / L \quad Y_E = P \alpha - (M_A - M_E) / L$$

$$M_A = -\frac{P \alpha L}{2} (\chi_1 - \chi_2) \quad M_E = -\frac{P \alpha L}{2} (\chi_1 + \chi_2)$$



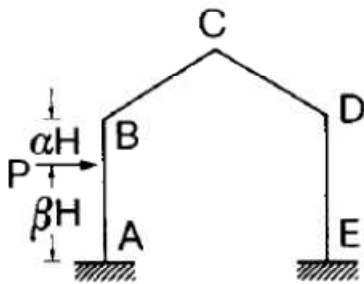
$$M_B = M_A + X_A H$$

$$M_C = M_E + X_A H (1 + \phi) - Y_E \frac{L}{2}$$

$$M_D = M_E + X_A H$$

$$M_0 = -M_A - X_A H (1 + 2\phi \alpha) + Y_A \alpha L$$

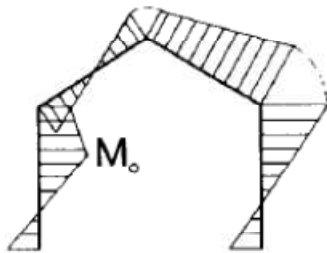
2.



$$\chi_1 = \frac{1}{K_2} [K_1 (4 + K_1 - 2\beta K_1 - 6\beta + 6\varphi) + \beta^2 K_1 (K_1 + 2 + \varphi) + 2\varphi K_1 (2\varphi - \beta\varphi - 3\beta) + \varphi^2]$$

$$\chi_2 = \frac{2 + 3K_1 (2 - \beta)}{2 + 6K_1}$$

$$X_E = \frac{P\beta}{2} \frac{1}{K_2} [6K_1 (1 + \varphi) + 6\alpha\beta K_1^2 + 4\beta^2 K_1 (1 + K_1) + -3\beta K_1 (2 + K_1) - 3\varphi K_1 (1 + \alpha)]$$



$$X_A = X_E - P$$

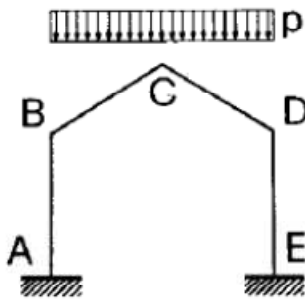
$$Y_E = -Y_A = P\beta \frac{H}{L} - \frac{M_A - M_E}{L}$$

$$M_A = \frac{P\beta H}{2} (\chi_1 + \chi_2) \quad M_B = \frac{P\beta H}{2} (\chi_1 - \chi_2)$$

$$M_C = M_E + X_E H (1 + \varphi) - Y_E \frac{L}{2}$$

$$M_D = M_E + X_E H \quad M_0 = -M_A - X_A \beta H$$

3.



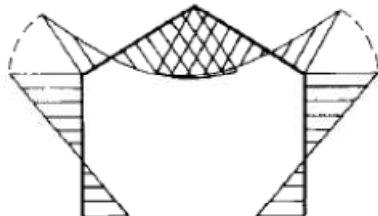
$$X_A = X_E = P \frac{L^2}{8H} \frac{1}{K_2} [K_1 (4 + 5\varphi) + \varphi]$$

$$Y_A = Y_E = P \frac{L}{2}$$

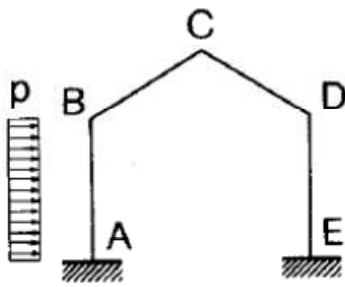
$$M_A = M_E = -P \frac{L^2}{48} \frac{1}{K_2} [K_1 (8 + 15\varphi) + \varphi (6 - \varphi)]$$

$$M_B = M_D = M_A + X_A H$$

$$M_C = -P \frac{L^2}{8} + M_A + X_A H (1 + \varphi)$$



4.

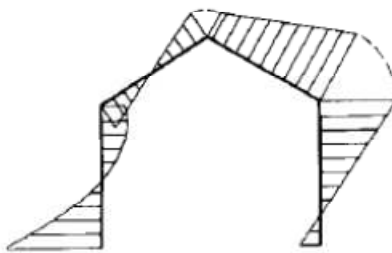


$$\chi_1 = \frac{1}{K_2} [K_1 (6 + K_1) + K_1 \phi (15 + 16\phi) + 6 \phi^2]$$

$$\chi_2 = (12K_1 + 6) / (1 + 3K_1)$$

$$X_E = p \frac{H}{4} \frac{1}{K_2} [K_1^2 + K_1 (3 + 2\phi)] \quad X_A = X_E - p H$$

$$Y_E = -Y_A = p \frac{H^2}{2L} - \frac{M_A - M_E}{L}$$



$$M_A = p \frac{H^2}{24} (\chi_1 + \chi_2)$$

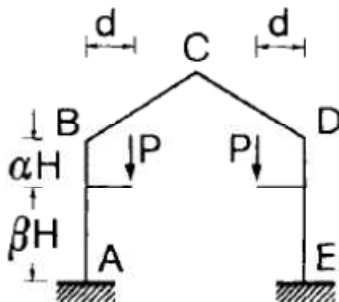
$$M_E = p \frac{H^2}{24} (\chi_1 - \chi_2)$$

$$M_B = M_A - p \frac{H^2}{2} + X_E H$$

$$M_D = M_E + X_E H$$

$$M_C = M_E + X_E H (1 + \phi) - Y_E \frac{L}{2}$$

5.



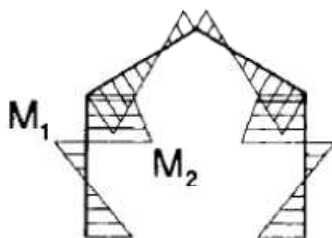
$$X_A = X_E = 6 \frac{Pd}{H} \frac{1}{K_2} [K_1 (1 + \phi + \alpha) - \alpha (\alpha K_1 + \alpha + \phi)]$$

$$Y_A = Y_E = P$$

$$M_A = M_E = -Pd \frac{1}{K_2} [K_1 (2\alpha K_1 + 2 + 3\phi) - \alpha \phi K_1 \cdot (6 + 3\alpha + 4\phi) - (3\alpha^2 K_1^2 + 6\alpha^2 K_1 + \phi^2)]$$

$$M_B = M_D = M_A - Pd + X_A H$$

$$M_C = M_A - Pd + X_A H (1 + \phi)$$

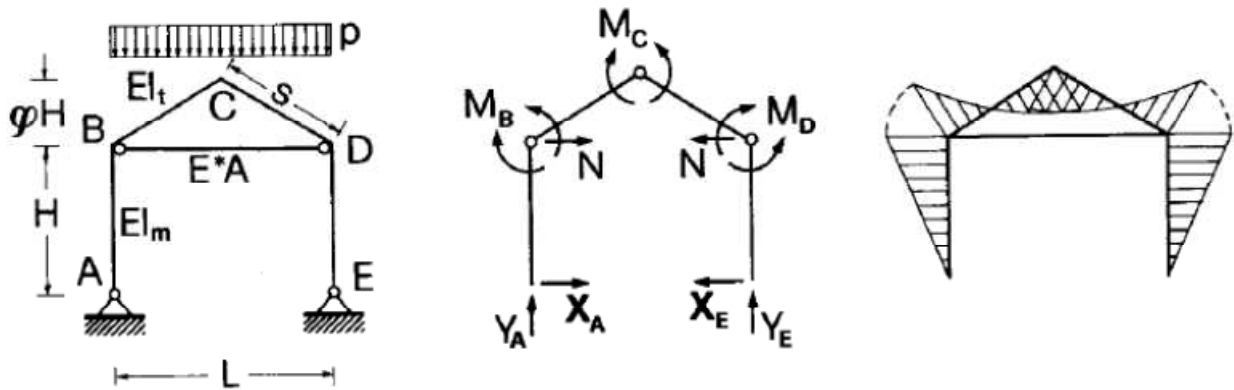


$$M_1 = M_A + X_A \beta H$$

$$M_2 = -M_A + Pd - X_A \beta H$$

f) SUPPORTED FRAME WITH TENSIONING HORIZONTAL BEAM

1.



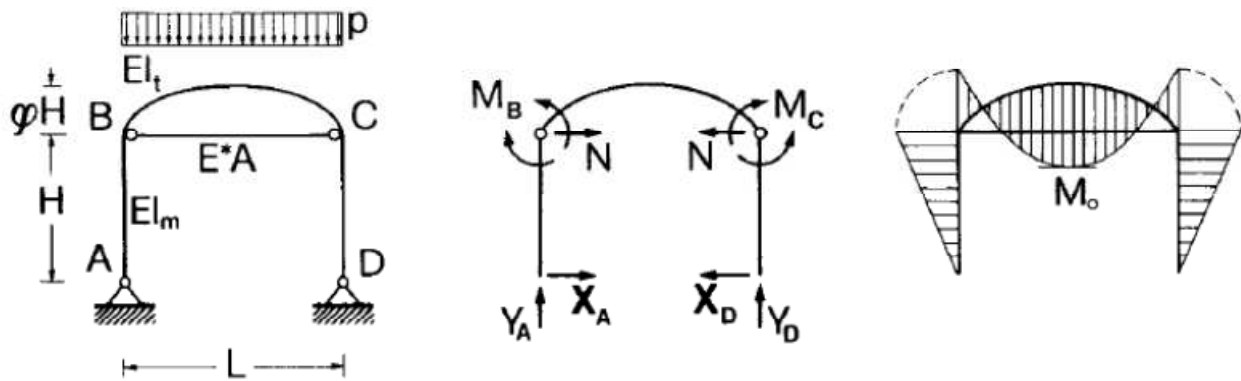
$$K_1 = \frac{H}{S} \frac{I_t}{I_m} \quad K_2 = \left[1 + \frac{3}{2} \frac{E I_t}{E^* A} \frac{L}{\varphi^2 H^2} \right]^{-1} \quad \lim_{E^* A \rightarrow \infty} K_2 = 1$$

$$X_A = X_E = p \frac{L^2}{16H} \frac{(16 - 15K_2) + 10 \varphi (1 - K_2)}{(4K_1 + 12 - 9K_2) + 12\varphi (1 - K_2) + 4\varphi^2 (1 - K_2)} \quad Y_A = Y_E = p \frac{L}{2}$$

$$N = \frac{5}{32} p \frac{L^2}{\varphi H} K_2 - \left(\frac{3}{2} \frac{1}{\varphi} + 1 \right) K_2 X_A$$

$$M_B = M_D = X_A H \quad M_C = -p \frac{L^2}{8} + X_A H (1 + \varphi) + N H \varphi$$

2.



$$K_1 = \frac{H}{L} \frac{I_t}{I_m} \quad K_2 = \frac{5 + 4 \varphi}{5 (3 + 2K_1) + 4\varphi (5 + 2\varphi)} \quad K_3 = 1 - 2 \varphi K_2$$

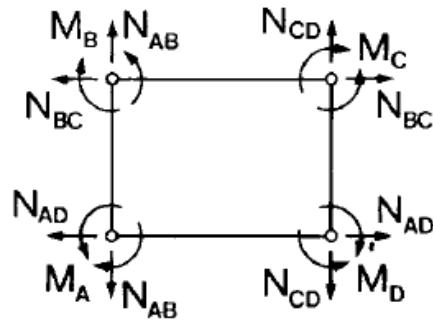
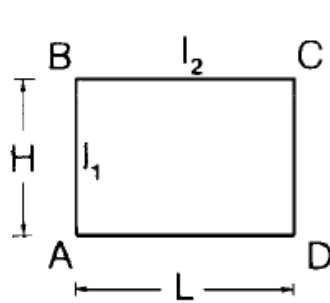
$$K_4 = \frac{E I_t}{E^* A} \frac{1}{\varphi^2 H^2} \cdot \frac{1}{K_2^2 (3 + 2K_1) - 2K_2 K_3 + 0,4K_3^2} \quad \lim_{E^* A \rightarrow \infty} K_4 = 0$$

$$X_A = X_D = p \frac{L^2}{4H} \frac{K_2 K_4}{1 + K_4} \quad Y_A = Y_D = p \frac{L}{2} \quad N = \frac{pL^2}{8H \varphi} \frac{1}{1 + K_4}$$

$$M_B = M_C = X_A H \quad M_0 = p \frac{L^2}{8} - X_A H (1 + \varphi) - N H \varphi$$

g) CLOSED RECTANGULAR FRAME

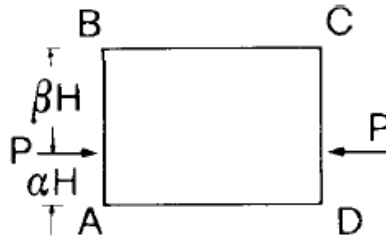
—



$$K_1 = \frac{H}{L} \frac{I_2}{I_1}$$

Legenda

1.



$$K_2 = (3 + 2K_1) + K_1 (2 + K_1)$$

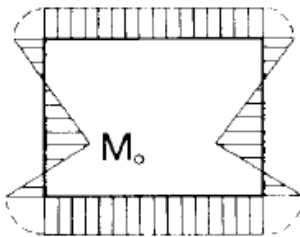
$$M_B = M_C = \frac{PH \alpha \beta}{3} \frac{K_1}{K_2} [(1 + \alpha) (3 + 2K_1) - K_1 (1 + \beta)]$$

$$M_A = M_D = \frac{PH \alpha \beta}{3} \frac{K_1}{K_2} [(1 + \beta) (3 + 2K_1) - K_1 (1 + \alpha)]$$

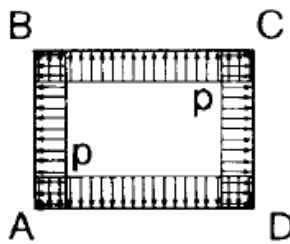
$$M_0 = PH \alpha \beta - M_A \beta - M_B \alpha$$

$$N_{AD} = -P \beta - \frac{M_A - M_B}{H} \quad N_{AB} = N_{CD} = 0$$

$$N_{BC} = -P \alpha + \frac{M_A - M_B}{H}$$



2.



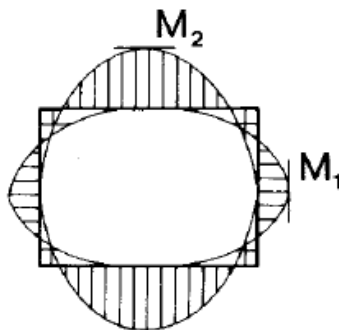
$$M_A = M_B = M_C = M_D = -\frac{p}{12} \frac{L^2 + H^2 K_1}{1 + K_1}$$

$$M_1 = p \frac{H^2}{8} + M_A$$

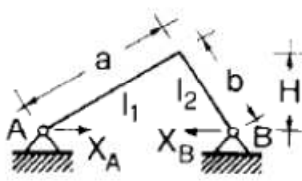
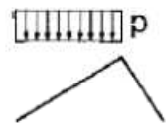
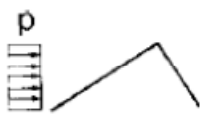
$$M_2 = p \frac{L^2}{8} + M_A$$

$$N_{AB} = N_{CD} = p \frac{L}{2}$$

$$N_{AD} = N_{BC} = p \frac{H}{2}$$



h) SHED TYPE FRAME

		$X_A = X_B = \frac{pL_1^2}{8HL} \frac{4L_2 + K(L + 4L_2)}{1 + K}$
<p>Legenda</p> $K = \frac{a}{b} \frac{I_2}{I_1}$		$X_A = X_B - pH$ $X_B = \frac{pH}{8L} \frac{4L_2 + K(L_1 + 5L_2)}{1 + K}$